

THE INFLUENCE OF CONCRETE STRENGTH ON THE ECONOMY OF
REINFORCED CONCRETE COLUMNS

by

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Dedication

This work is dedicated to my father,
my mother and my sweet heart, Samira,
for their love and care throughout my academic life.

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THE INFLUENCE OF CONCRETE STRENGTH ON THE ECONOMY OF REINFORCED CONCRETE COLUMNS

ABSTRACT

The effect of increase in strength of concrete on the economy and performance of reinforced concrete columns is investigated. Two different concrete strengths namely 25 and 45 MPa were used to design columns of 16 different buildings with variation in floor panel size and number of stories carried by the column.

The study indicates that, improvement on the strength of concrete causes a substantial decrease in the size and / or the reinforcement of columns which in turn lowers the cost of reinforced concrete frames. The decrease in cost (savings) is observed to increase with increase in number of stories carried .

The cost benefit in reinforced concrete columns from increasing concrete strength decreases as the floor panels get larger.

BETON DAYANIMIN BETONARME KOLON EKONOMİSİNE ETKİSİ

ÖZET

Bu çalışmada artan beton basınç dayanımının betonarme kolonlar üzerindeki ekonomi ve performans etkileri incelenmiştir. 25 ve 45 Mpa dayanımlı iki beton, kat boyutlarında farklılık gösteren 16 değişik bina dizaynı için kullanılmıştır.

Araştırma neticesinde dayanımdaki artışın kolon boyutlarını ve donatı miktarını azaltarak betonarme yapıların maliyetini düşürdüğü ortaya çıkmaktadır. Artan kat sayısı, maliyette daha büyük azalmalara sebep olmaktadır.

Kolonlarda kullanılan beton dayanımının artması ile elde edilen ekonomik kazançta, döşeme panel açıklıkları arttıkça azalma gözlenmiştir.

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1. INTRODUCTION

Today, concrete is commonly used in construction industry, especially in building frames; and it replaces steel in constructing most tall buildings, due to the recent improvement in its strength.; thus by using high – strength concrete.

The term high – strength in the concrete industry simply describes the compressive strength of concrete at 28 days above a certain threshold, which is relative, and depends on the place and time of consideration. For instance, for almost a century, structural concrete has been routinely produced at strengths ranging from 20 to 30 MPa. Four decades ago, in Chicago a 41 MPa was first produced and labeled a high strength, and more recently the threshold rose to 50 – 60 MPa. However, due to high demand in high – rise buildings and bridges, concrete of much higher strength have entered the field of construction: 90, 100, 110 MPa and even higher.

Since both normal and high strength concrete are made up of the same ingredients namely, Portland cement, aggregates and water, what make the difference between the two are the mix proportion, technical know – how, and some differences in the ingredients, in that, high strength concrete invariably contains a high range of superplasticizer while normal concrete does so only sometimes.

High strength concrete with high modulus of elasticity, high density, and low permeability has been used for various reasons including: resistance to freezing and thawing effect, alkali – aggregate reaction, corrosion, leaching, abrasion and chemical attack; all of which decrease as the water cement ratio w/c decreases. But this work is limited to the investigation of cost of reinforced concrete columns as related to concrete strength. Floor panel carrying elements include both columns and walls, only columns are considered here. The same economic advantages apply to high strength in walls as in columns, with the added advantage that a saving in the wall area generally improves the net rentable area.

The aim of this work is to find out how the increase in concrete compressive strength influences the cost of column in reinforced concrete frames. To investigate this, two main variables have been chosen namely the number of column stories and the dimension of floor panels which the columns carry. All other factors are kept constant.

Change in the first variable alters the normal force whereas that of the second alters the moments as well as the normal force in the column.. Sixteen different cases of buildings

were analyzed using 25 and 45 MPa concrete, having two square floor panel sizes and four different stories.

Three dimensional structural analysis of the buildings were performed in both the transverse and longitudinal directions for the lateral load cases combined with the gravitation loads using SAP90; and the result served as the input data for reinforcement design software which in turn gives the total steel used for each column.

Cost analysis is made using the June 1998 Turkish prices for concrete and reinforcing steel, and results compared for different sets of buildings.

2. MATERIALS

2.1 Concrete Production

Concrete is made up of inert aggregate particles bounded by Portland cement-water paste. A number of standard Portland cements exists and are used according to the desired strength of Reinforced Concrete.

The aggregate which consists of both fine and course particles, exerts a major influence on bleeding, ease of finishing, shrinkage and amount of cement and water required to make a given unit of concrete of a given consistency. Importance is also given to the water – cement ratio since that is the major determinant of concrete strength.

In the concrete design practice, proper curing of the concrete is very essential so that water in the mix does not evaporate before the concrete gains the desired strength.

2.2 Strengths of Concrete

Concrete generally has high compressive strength and relatively low tensile strength usually between 10 to 15% of the compressive strength. The shear strength of concrete is however large, approximately 35 to 80% of the compressive strength [1].

2.3 Formation of Reinforced Concrete

The weakness in tension of the plain concrete makes it economical only for footings, concrete slabs laid on the ground and for massive structures such as retaining walls; even then reinforcement is frequently employed.

It is however uneconomical to accept the low tensile strength of plain concrete as a unit on structural strength. The use of reinforcing steel bars in concrete improves markedly upon the economy of construction in that steel, having predominantly high tensile strength carries entirely, the tensile stresses exposed to the structure. [1]

Although steel occupies only a small part of the volume of reinforced concrete (approximately 1 %), it is a major part of the cost. As a very rough guide, the cost of the formwork, the cost the concrete and the cost of the steel are approximately the same, that is one – third of the total cost.

Since reinforcing steel rusts very readily, adequate concrete cover to ensure durability of structure must protect it. The steel is therefore completely surrounded by the concrete; thus, the strain can be transmitted to the steel only via the concrete. The proper adhesion between the steel and the concrete is of greatest importance, and bars should be of sufficiently small diameter to offer an adequate area of contact with the steel.

3. HIGH STRENGTH CONCRETE

3.1 Formation of High Strength Concrete

The compressive strength of concrete is generally governed by the strength of either the cement paste or of the bond between the paste and the aggregate particles.

The strength of cement paste in turn, to a large extent depends on the water – cement ratio and the degree of hydration.

For a given cement and acceptable aggregates, the strength that may be developed by a workable, properly placed mixture of cement, aggregate and water is influenced by the:

- Ratio of mixing water to cement
- Ratio of cement to aggregate
- Grading, surface, texture, shape, strength and stiffness of aggregate particles
- Maximum size of aggregate
- Type and brand of cement
- Amount and type of admixture or Pozzolan
- Mineralogical makeup of the aggregate.

3.2 Development and Technology of High Strength Concrete

In the 1950s, only natural aggregates (sand, gravel), ordinary Portland cements or slag – cements and water were used as constituents for concrete production. Water was necessary for sufficient workability, and the water – cement ratio w/c was normally ranging between 0.6 and 0.7, which limited the compressive strength to about 20 – 40 MPa.

The advent and utilization of water reducing agents and superplasticizers in the concrete industry during the 70s, decreased the w/c – ratio range to about 0.45 – 0.60 without any negative effect on the concrete's workability, and the compressive strength improved to about 60–70 MPa.

In order to obtain yet higher concrete strengths, the w/c ratio was reduced further to 0.25, using some extremely effective super plasticizers, called High Water Reducing Agents (HWRA).

Since the concrete strength is governed not only by the w/c – ratio, but also the cement strength, it was not possible to exceed a concrete strength of 100 MPa by the reduction of the w/c ratio alone; instead, another agent called silica fume was added. [2]

3.3 Mix Proportions of High Strength Concrete

What makes the difference between normal strength and high strength concrete is basically the know – how. The ingredients of both types of concrete are the same, namely, Portland cement, aggregate, water and admixture. There is however a slight difference in the ingredient in that high strength concrete invariably contains a high range of water reducer (superplasticizer), while normal concrete does so only sometimes [3]. In practice, the mix proportions vary, depending on the properties of the individual ingredients and the desired properties of the concrete in service. Details of some actual mixes used in the past are shown in table 3.3 [4].

Usually, Type I cement (in ASTM classification) is used but, if high early strength is required, Type III cement can be used. As shown in the table, other cementitious materials are also included in the mix making the total content cementitious materials to be as high as: 400 to 550 Kg/m³.

To achieve a high strength, the aggregates should not have too large a maximum size. The coarse aggregate is required to be strong, absolutely clean, that is free from adhering clay or dust; it must not contain reactive silica; and it must be equidimensional in shape, that is neither flanky nor elongated. As for fine aggregate, it is required to be coarsely graded, preferably with a fineness modulus of 2.7 to 3.0. [4]

Inclusion of supplementary cementitious materials, mainly fly ash and slag is very common in the normal strength concrete because of their economic importance in that they are generally cheaper than Portland cement and also, they hydrate or react chemically somewhat later than Portland cement. In the case of the high strength concrete, the reasons for their use are stronger. Given that the high strength is due, in part, to the presence of a dense matrix, replacing a portion of the Portland cement with the supplementary cementitious materials would not unduly depress the early strength of the concrete [3]. Also, the lower chemical reactivity of supplementary cementitious materials means that a

partial replacement of cement is beneficial from the standpoint of controlling the rheological properties of the high strength concrete [3].

Water cement ratio is a very critical issue in the mix since it is the major determinant of the concrete strength in addition to the density of the resulting matrix and the strength of aggregate. The combination of the two major effect namely the w/c and density of the matrix provides a situation in which the w/c influences strength only above a certain minimum value of the ratio. It has been found that the optimum value of w/c is about 0.22. At values higher than 0.22, the influence of w/c is operative; lower values are however harmful because an adequately high density of the matrix cannot be achieved. As seen in Table 3.3, water – cementitious material ratio, $w/(c+m)$, is used rather than the usual water-cement ratio, w/c.

Generally speaking, the cement hydrates more rapidly than supplementary cementitious materials and therefore takes up most of the mixing water. Because of this, it is tempting to conclude that the density of the matrix at the early stage of hardening is governed by w/c, regardless of the other finely divided particles. [3]

However, recent experimental work, as well as field observations show that the finer particles of blast furnace slag, and fly ash all enter into the hydration reactions before some coarser cement particles have done so. It is therefore concluded that it is preferable to quote w/c and $w/(c+m)$ and /or the water content of the mix [3].

Table 3.3 – Mix Proportions of some High Strength Concrete[3]

Ingredient (Kg/m ³)	Mix								
	A	B	C	D	E	F	G	H	I
Portland Cement	534	500	315	513	163	228	425	450	460
Silica fume	40	30	36	43	54	46	40	45	—
Fly ash	59	—	—	—	—	—	—	—	—
Slag	—	—	137	—	325	182	—	—	—
Fine aggregate	623	700	745	685	730	800	755	736	780
Coarse aggregate	1069	1100	1130	1080	1100	1110	1045	1118	1080
Total water	139	143	150	139	136	138	175	143	138
w/c ratio	0.22	0.27	0.31	0.25	0.25	0.3	0.38	0.29	0.3
Slump,mm	255	—	—	—	200	220	230	230	110
Cylinder strength (Mpa) at age (days)									
1	—	—	—	—	13	19	—	35	36
2	—	—	—	65	—	—	—	—	—
7	—	—	67	91	72	62	—	68	—
28	—	93	83	119	114	105	95	111	83
56	124	—	—	—	—	—	—	—	—
91	—	107	93	145	126	121	105	—	89
365	—	—	—	—	136	126	—	—	—

3.4 Evolution of High Strength Concrete

To satisfy developer's need for higher strength structures and maintaining column size to attain uniform and larger rentable floors, the concrete construction industry had been striving to improve upon the concrete strength.

In 1962, Material Service Corporation, in Chicago, for the first time, then subsequently developed and delivered to job sites the strengths as shown in Table 3.4. [5]

Table 3.4- High Strength concrete delivered for the first time to Chicago project [5]

1962	Outer Drive East	6000 Psi	41 MPa
1965	Lake Point Tower	7500 Psi	52 MPa
1972	Mid- Continental Plaza	9000 Psi	62 MPa
1976	River Plaza	11000 Psi	76 MPa
1982	Chicago Mercantile E.	14000 Psi	96 MPa
1988	Construction Technology Laboratory	17000 Psi	117 MPa

3.5 Utilization of High Strength Concrete

The first High Strength Concrete structure was a high - rise building in the USA. The highest concrete strength ever used till today was in the construction of a 220m high "Two-Union Square in Seattle, in 1988. Four 3m diameter interior columns and fourteen 1m-diameter columns within the facade both of BS140 strength carried the structure. This concrete was cast in steel pipes, which were used as form work as well as hoop reinforcement. To ensure a complete filling of the pipes without any external compaction, the fresh concrete was pumped from the bottom to the top over two floors in each step. For this purpose, a very weak consistency with a flow value of $a=64$ to 76 cm was required. Table 3.5.1 gives the mix composition of that. [2]

In 1990, a 186m high - rise building "Trianon" of BS85 strength concrete was constructed for the first time in Frankfurt, Germany.

Table 3.5.1- Concrete mix for High Strength concrete BS140
(Two Union Square, Seattle)

Cement ASTM Type 2	560 Kg/m ³
Silica fume	42 Kg/ m ³
Sand 0/5	650 Kg/ m ³
High Strength Aggregates 5/10	1110 Kg/ m ³
Water	140 Kg/ m ³
Superplasticizers	9 – 14 Kg/ m ³
Retarder	3 l/ m ³
W/C ratio	0.23
Flow	64 – 76 cm
Modulus of elasticity	49,700 N/mm ²

Today in Germany, up to a BS115 strength is applied in various projects on site as well as in prefabricated elements [2]. For flexural and shear - stress members in the high-rise building "Taunustor" in Frankfurt, Germany, a High Strength Concrete of class BS105 was used for the first time [2].

Other prominent structures were the Water Tower Place in Chicago (1975), Joigny Bridge in France (1989), La Laurentienne Building in Montreal (1984) and the Scotia Plaza in Toronto (1987). Table 3.5.2 summarizes the mixtures for these structures.

Table 3.5.2 – Typical High strength concrete mixtures

Mixture number		1	2	3	4	5
Water	Kg/ m ³	195	165	135	145	130
Cement	Kg/ m ³	505	451	500	315	513
Fly Ash	Kg/ m ³	60	---	---	---	---
Slag	Kg/ m ³	'---	---	---	137	---
Silica fume	Kg/ m ³	'---	---	30	36	43
Coarse aggregate	Kg/ m ³	1030	1030	1100	1130	1080
Fine aggregate	Kg/ m ³	630	745	700	745	685
Water reducer	ml/ m ³	975	---	---	900	---
Retarder	L/ m ³	---	405	1,8	---	---
Superplasticizers	L/ m ³	---	11,25	14	5,9	15,7
w / (c+m)		0,35	0,37	0,27	0,31	0,25
f _c at 28 day	(MPa)	65	80	93	83	119
f _c at 91 day	(MPa)	79	87	107	93	145
1- Water Tower Place, Chicago (1975)						
2- Joigny Bridge, France (1989)						
3- La Laurentienne Building, Montreal (1984)						
4- Scotia Plaza, Toronto (1987)						
5- Two Union Square, Seattle (1988)						

3.6 Properties of High Strength Concrete

3.6.1 Stress - Strain Relation

The difference between normal strength concrete and high strength concrete are not only in the increased strength but also in different stress – strain relation and consequently, the elastic modulus.

In high strength concrete, the stress – strain curve is nearly linear up to a stress strength ratio of about 80%. After exceeding the ultimate load, the stress – strain line falls off rapidly, which is typical for a brittle material. Using increased stirrup reinforcement can prevent such a sudden failure of a structure. [2]

Corresponding to the steeper stress-strain line, the modulus of elasticity of high strength concrete is also higher than that of normal concrete. The increase in elastic modulus, however, is less than the increase in compressive strength.[2]

3.6.2 Ductility

Generally, plain concrete is not a ductile material. Plain high – strength concrete is less ductile than that of a lower strength concrete, although the relative ductilities of the two materials when reinforced is less well understood.[6]

The approach used to improve the ductility of concrete columns is substantially the use of additional tie reinforcement. Thus increasing the volume of ties in the column can provide a much more ductile column. If sufficient ties are used, substantial residual strength will remain after the outside shell of the concrete has completely spalled off. Apparently, spalling of the concrete occurs more easily in higher strength concrete because of their lower water content. [6]

3.6.3 Durability

For many years, the incentive for developing high-strength concrete was essentially strength oriented. While the ultimate strength of production of concrete has tripled over the past four decades, the durability of concrete has not improved as much.

Almost all aspects of durability are related to the permeability of concrete, which generally decreases with increase in strength. The use of high-strength concrete could therefore solve many durability problems such as: freezing and thawing, alkali-aggregate reaction, corrosion, leaching, abrasion and resistance to chemical attack which decreases as the w/c decreases. [7]

3.6.4 Fire resistance

Due to its brittleness, high-strength concrete normally has a very low fire resistance. At temperatures between 800-1000 degree C, water in the cement paste which is physically as well as chemically bonded is set free and converted to steam. In normal concrete, the steam can escape or at least the steam pressure can be released due to its adequate capillary pores; whereas in high strength concrete, the dense microstructure of the cement paste prevents such a pressure reduction which would result in a sudden bursting of the concrete cover, so that the reinforcement is subject to direct fire and high temperatures without any protection, resulting in the failure of the structure. [2]

3.7 Regulations on the Use of High Strength Concrete

Although the American code does not have specific upper limits on concrete strength, other parts of the world have strict regulations and limitations on the use of high strength concrete. The current Australian concrete structures code, AS3600, recognizes only the use of concrete up to 50 MPa in strength, although a modification allowing the use of 60 MPa is expected in the near future. [6]

In 1990, high-strength concrete of class BS85 was not regulated in German Standards, so a special permit with extensive investigation was required for the structural

application of BS85 concrete in columns and walls for the 186m high rise building “Trianon” in Frankfurt. Today in Germany, high-strength concrete is applied up to strength of 115 MPa in various projects. [2]

3.8 When to Use High Strength Concrete

The most common requirement for high strength is in compression members. Here, high strength allows the use of smaller columns and therefore, a reduction in weight and hence, a lower load on the foundation. Also a smaller part of the horizontal area is occupied by columns so that there is more of the economically valuable floor space.

Compared with normal reinforced concrete structures, high strength concrete structures have advantages in the smaller dimensions of the members and / or in a reduced reinforcement. For instance, a column 1 x 1m using normal concrete of strength 45 MPa and reinforcement $\mu = 4\%$, can carry a load of 21 MN. This same load can be carried by an 85 MPa high strength concrete column of cross section 1 x 1 m with $\mu = 1.23\%$ of reinforcement. Besides the saving in reinforcement, the concrete placement is facilitated.

4 SOME UNDESIRABLE PHENOMENA IN CONCRETE STRUCTURES

4.1 Creep and Shrinkage

Concrete creep is the deformation and flow of matter associated with structure of the cement paste whilst the shrinkage and swelling of concrete is produced by the movement of water in to and out of the gel due to external load and drying.

The creep in concrete is caused by mixture proportions, type of aggregate and age at loading whilst the shrinkage is directly related to water- cement ratio, mixture proportions, type of aggregate and curing condition. [8]

4.2 Influence of Constituent on Creep and Shrinkage

An experimental investigation was conducted on high strength concrete produced in the St. Paul – Minneapolis area which reveals that by lowering the paste content and maximizing the coarse aggregate, a high strength concrete can be produced with a modest improvement in creep and shrinkage deformations. [8]

4.3 Effect of Creep and Shrinkage on the Economy of Structure

The creep increases with the increase in both the cement and water content of the concrete per unit volume. Since concrete required to carry high stresses is frequently as rich as 4:1, it consequently suffers from both large shrinkage and creep movements. The reduction of column length due to these two phenomena can result in the partial transfer of load from the columns to the cladding of the structure, which begins to crush since it is usually not designed to carry load. Overcoming this negative effect is achieved by forming horizontal construction joints at a number of levels with a subsequent increase in building cost. [9]

4.4 Creep and Shrinkage in High –Strength and Normal Strength Concrete

Studies on the creep and shrinkage show advantages of high-strength over normal strength concrete as summarized below:

- Although high- strength concrete generally have greater unit cement paste volume, its drying shrinkage is smaller than that of normal strength. This is because, beside other factors, the drying shrinkage is dependent on the unit cement paste volume and the compressive strength of the concrete.[10]
- For a high- strength concrete of strength higher than 98 MPa, the value of specific creep divided by the unit cement paste is $30.6 \sim 40.8 \times 10^{-6} / \text{MPa} / \text{m}^3 / \text{m}^3$ both under water and in air which is $\frac{1}{4} \sim \frac{1}{5}$ that of normal strength concrete under air curing. Also the difference in creep between concrete cured in the air and that cured in water diminishes with increase of compressive strength, and does not exist for high – strength concrete with compressive strength over 98 MPa.[10]

5. GENERAL DESIGN CONSIDERATION

5.1 Type and System of structure

The selection of system type highly depends on the function of the building under consideration since this give us the idea about the size of the open space required, mechanical services and the need for flexibility. In this study, residential buildings including apartment, hotels and dormitories which are frame structurally more suitable are considered.

5.2 Slab System

The choice of a slab system for vertical loads depends upon the length of span, loading intensity and the function of the building.

The need for smooth ceiling in residential occupancies resulted in the flat plate being the most prevalent and economical [11]. Therefore, plate floor system is used for frame analysed.

For the flat plate two-way slab system, the minimum thickness, h , as given in codes is:

$$h = \frac{l_n}{36000} \left(800 + \frac{f_y}{200} \right) \quad (5.2.1)$$

Where l_n is the length of the clear span in the long direction and f_y is the yield stress of the steel in Psi [12,13].

Using the equivalent frame method of design, the flat plate and its supporting columns are considered as a series of elastic frames at right angles to one another [12]. Each of these frames are analysed as if it were a frame consisting of beams and columns.

5.3 Columns

The most common cause of the major damage to buildings is the insufficient ductility in the bearing walls and columns. By suitable reinforcement, ductility is introduced in to the reinforced concrete structures.

Since the cost of reinforcing bars is predominantly high, the amount of reinforcement has a very significant effect on the economy of high rise reinforced concrete buildings. The lowest possible percentage of vertical reinforcement is sometimes attempted for some economic reasons . This can be achieved by increasing the column size (if there is no conflict with the architectural requirement), by increasing the strength of concrete used or by increasing both the strength and the column size.

Quite often, in the upper stories of buildings, columns are much larger than required to carry the load. The oversized columns result either from architectural requirement or from economical considerations to have a few changes in column section.

5.4 Geometric Parameters

The main variables considered are the number of stories and the span lengths in square floor panels shown in fig. 5.4

Two sets of floor spans namely 5m by 5m and 7m by 7m were chosen to be designed each for 5, 10, 15 and 20 stories using 25 MPa and 45 MPa concrete strengths. For a typical representative of concrete frame structures, a 3m-column height was chosen.

After some preliminary calculations, the corresponding slab thickness for various span lengths were obtained as:

5m by 5m \longrightarrow 16 cm

7m by 7m \longrightarrow 22 cm

The slab-beam have the same dimensions at all floor levels whilst the columns have varying dimensions. Tables 5.4.1- to 5.4.5 show the preliminary columns and slab-beams cross sections

Table 5.4.1 – Beams cross sections

Bay size	for 25 Mpa Concrete	for 45 Mpa Concrete
5m square	500 by 250 mm	400 by 250 mm
7m square	600 by 350 mm	500 by 300 mm

Table 5.4.2 – Column cross sections for 20 story buildings

fc= 25 Mpa					fc= 45 Mpa				
Story level	5m span		7m span		Story level	5m span		7m span	
	Int.	Ext.	Int.	Ext.		Int.	Ext.	Int.	Ext.
1	900	700	1100	900	1	700	600	900	800
2	900	700	1100	900	2	700	600	900	800
3	900	700	1100	900	3	700	600	900	800
4	900	700	1100	900	4	700	600	900	800
5	900	700	1100	900	5	700	600	900	800
6	800	600	1000	800	6	600	500	800	700
7	800	600	1000	800	7	600	500	800	700
8	800	600	1000	800	8	600	500	800	700
9	800	600	1000	800	9	600	500	800	700
10	800	600	1000	800	10	600	500	800	700
11	700	500	900	700	11	500	400	700	600
12	700	500	900	700	12	500	400	700	600
13	700	500	900	700	13	500	400	700	600
14	500	400	700	600	14	400	300	600	500
15	500	400	700	600	15	400	300	600	500
16	500	400	700	600	16	400	300	600	500
17	400	300	600	500	17	350	300	500	400
18	400	300	600	500	18	350	300	500	400
19	400	300	600	500	19	350	300	500	400
20	400	300	600	500	20	350	300	500	400

Table 5.4.3- column cross sections for 5 story buildings

$f_c = 25 \text{ MPa}$					$f_c = 45 \text{ MPa}$				
Story	5m span		7m span		Story	5m span		7m span	
level	Interior	Exterior	Interior	Exterior	level	Int.	Exterior	Int.	Exterior
1	500	400	700	600	1	400	300	600	500
2	500	400	700	600	2	400	300	600	500
3	500	400	700	600	3	400	300	600	500
4	500	400	700	600	4	400	300	600	500
5	500	400	700	600	5	400	300	600	500

Table 5.4.4 - column cross sections for 15 story buildings

$f_c = 25 \text{ MPa}$					$f_c = 45 \text{ MPa}$				
Story	5m span		7m span		Story	5m span		7m span	
level	Interior	Exterior	Interior	Exterior	level	Int.	Exterior	Int.	Exterior
1	800	600	1000	800	1	600	500	800	700
2	800	600	1000	800	2	600	500	800	700
3	800	600	1000	800	3	600	500	800	700
4	800	600	1000	800	4	600	500	800	700
5	800	600	1000	800	5	600	500	800	700
6	700	500	900	700	6	500	400	700	600
7	700	500	900	700	7	500	400	700	600
8	700	500	900	700	8	500	400	700	600
9	700	500	900	700	9	500	400	700	600
10	700	500	900	700	10	500	400	700	600
11	600	400	800	600	11	400	300	600	500
12	600	400	800	600	12	400	300	600	500
13	600	400	800	600	13	400	300	600	500
14	400	300	600	500	14	350	300	500	400
15	400	300	600	500	15	350	300	500	400

Table 5.4.5- column cross sections for 10 story buildings

fc= 25 MPa					fc= 45 MPa				
Story	5m span		7m span		Story	5m span		7m span	
level	Interior	Exterior	Interior	Exterior	level	Int.	Exterior	Int.	Exterior
1	700	500	900	700	1	500	400	700	600
2	700	500	900	700	2	500	400	700	600
3	700	500	900	700	3	500	400	700	600
4	700	500	900	700	4	500	400	700	600
5	700	500	900	700	5	500	400	700	600
6	500	400	700	600	6	400	300	600	500
7	500	400	700	600	7	400	300	600	500
8	500	400	700	600	8	400	300	600	500
9	500	400	700	600	9	400	300	600	500
10	500	400	700	600	10	400	300	600	500

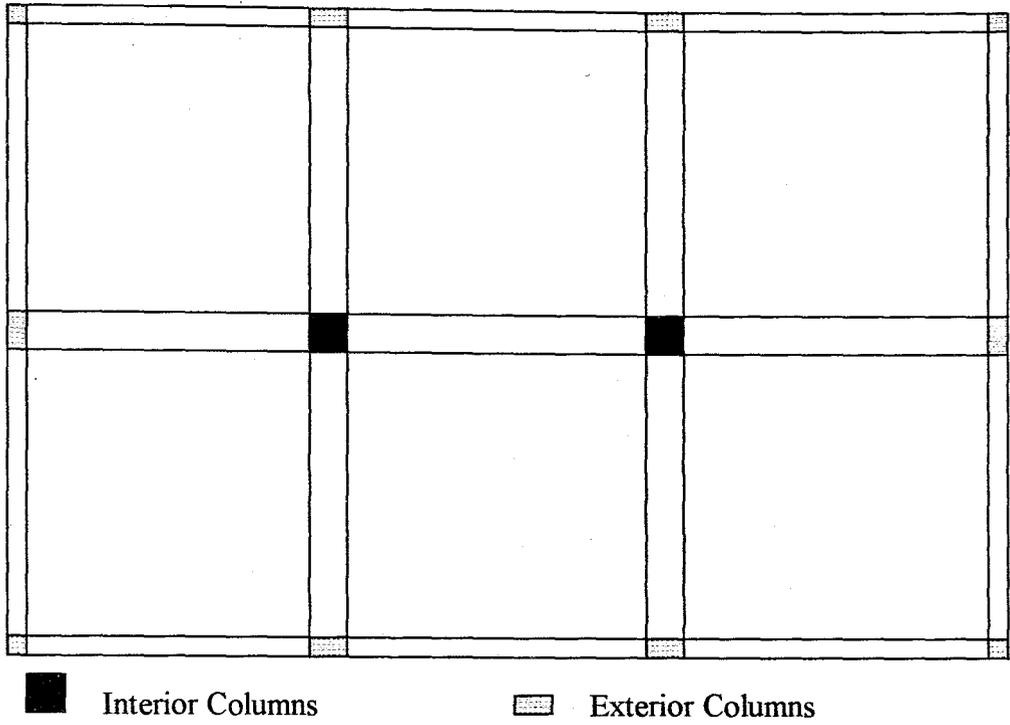


Figure 5.4.1 – Plan View of a Typical Square panel

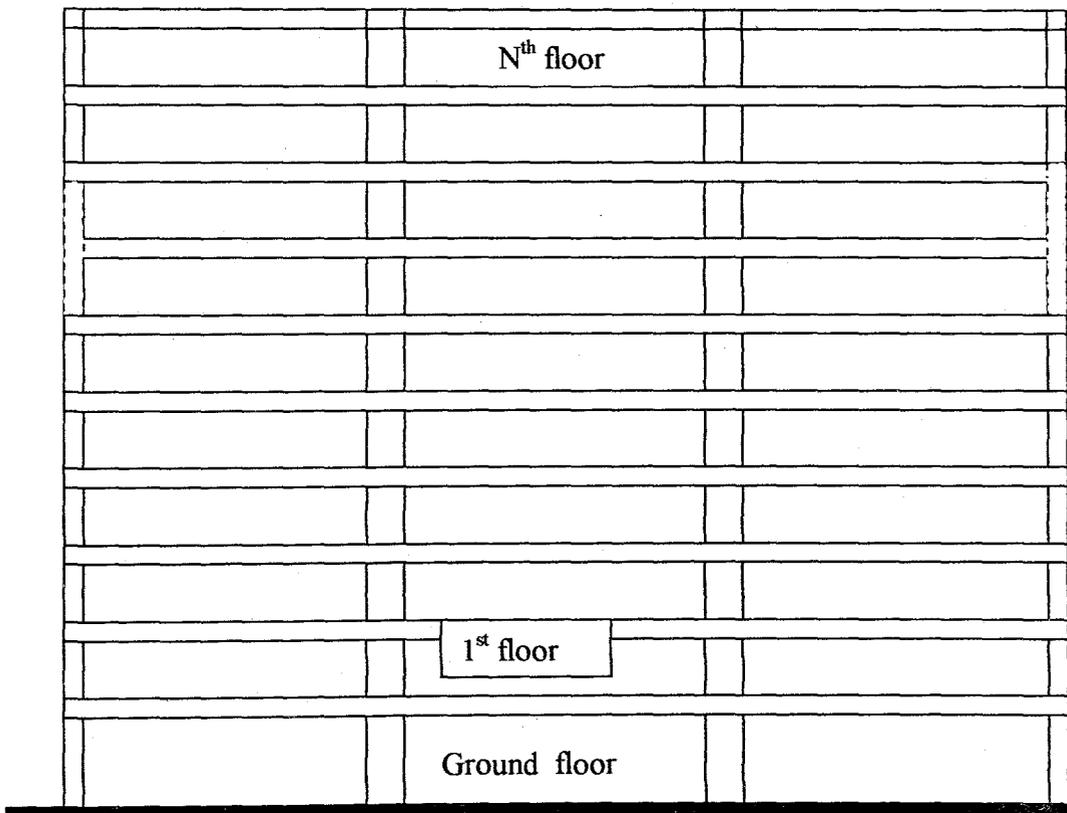


Figure 5.4.2 – Side View in the Long Direction of a typical N story

5.5 DESIGN OF LOADS

5.5.1 Gravity Loads

The basic aspect of engineering is the design of gravity load even for tall structures in effect on the economy of the project. Proper column layout which leads to economical spans while not impeding the functional requirements of the building is the key to an economical slab system to support the gravity loads.

The gravity loads considered are the service live loads (G) plus the superimposed dead loads (Q) obtained from partitions, ceilings and mechanical.

5.5.2 Horizontal loads

These are the lateral forces exerted on the building by wind (W) and / or earthquake (E). According to the design code, the wind and the earthquake (seismic) loads are not considered simultaneously in the design [12], as explained below:

If winds are considered:

1	$1.0G + 1.3Q + 1.3W$
2	$1.4G + 1.6Q$
3	$0.9G + 1.3W$

If seismic loads are considered:

4	$1.0G + 1.0Q + 1.0E$
5	$1.4G + 1.6Q$
6	$0.9G + 1.0E$

The code depicts that the combination with the most severe condition is selected.

After some preliminary calculations, combination number 4 was selected.

5.5.3 Design of Seismic Loads

Depending upon the building and the seismic zone or seismic performance category, the seismic forces were applied in the direction that produces the most critical load effect as stipulated in the design code [13] which states that:

The principal steps involved in the earthquake resistance design of a typical concrete structure according to building code provisions are as follows:

1. Determination of design earthquake forces:
 - a) calculation of base shear corresponding to computed or estimated fundamental period of vibration of the structure (a preliminary design of the structure is assumed here);
 - b) distribution of the base shear over the height of the building.
2. Analysis of the structure under the (static) lateral earthquake forces calculated in step 1, as well as under gravity and wind loads, to obtain member design forces.
3. Designing members and joints for the most unfavourable combination of gravity and lateral (wind or seismic) loads, and detailing them for ductile behaviour.

5.5.4 Design Base Shear

The design base shear represents the total horizontal seismic force that may be assumed to be acting parallel to the axis of the structure considered. The force in the other direction is assumed to act non-concurrently.

The requirement that orthogonal effects be considered in the proportionality of a structural element may be satisfied by designing the element for 100 percent of the prescribed seismic forces in one direction plus 30 percent of the prescribed forces in the perpendicular direction. The combination requiring the greater component strength must be used for design [13].

The total lateral force or base shear, V , is given by:

$$V = Z K C W \quad (5.5.4.1)$$

Where: Z is seismic zone coefficient

Taken as 1 for zone III [UBC]

K - factor of structural system and type ($k=1$ from table 12-3 [14])

C - flexibility coefficient

C values were obtained from charts [14]

W is the total dead load, except that in storage and warehouse occupancies.

With $k=1$ and $Z=1$, equation (5.5.4.1) becomes

$$V = C W \quad (5.5.4.2)$$

The lateral seismic design forces, F_x are obtained from the distribution of the base shear, and is given by the relation:

$$F_x = \frac{V w_x h_x}{\sum_{i=1}^n w_i h_i} \quad (5.5.4.3)$$

6. CALCULATIONS OF DESIGN LOADS

6.1 Design Data

In this work, design loads are calculated according to the 1994 edition of the Uniform Building Code (UBC)[13]. As shown in section 5 and tables 5.4.1 to 5.4.5, the floor panels have the same dimensions at all floor levels whilst the columns have varying dimensions. Other design data are as follows:

Service loads:

Live loads: 50 psf (UBC -94, Table 16-A)

Additional average value to allow for heavier load on corridors = 25 psf

Thus, total average live load = 75 psf (0.359 ton/m²)

Superimposed Average for partitioning = 20 psf

dead load : Ceiling and mechanical = 10 psf

Thus total average superimposed dead load = 30 psf (0.1536 ton/m²)

Material properties:

Concrete: $f_{c1} = 25$ MPa; $f_{c2} = 45$ MPa; $w_c = 2.323$ ton/m³

Reinforcement: $f_y = 460$ MPa

6.2 Calculations

Given that: $w_c = 2.323$ ton/m³ and

Dead load + Live load = 0.5126 ton/m²

a) For the 5m by 5mspan of thickness 16 cm, the total weight exerted by the slab is:

$$W = 0.5126 + 2.323 \times 0.16 = 0.88429 \text{ ton/m}^2$$

As shown in figure 6.2.1, each interior beam carries a wholespan area whereas, the exterior beams carry half the area of aspan each. Thus,

$$\text{Load on the Interior beams} = 0.88429 \times 5 = 4.421 \text{ ton/m}$$

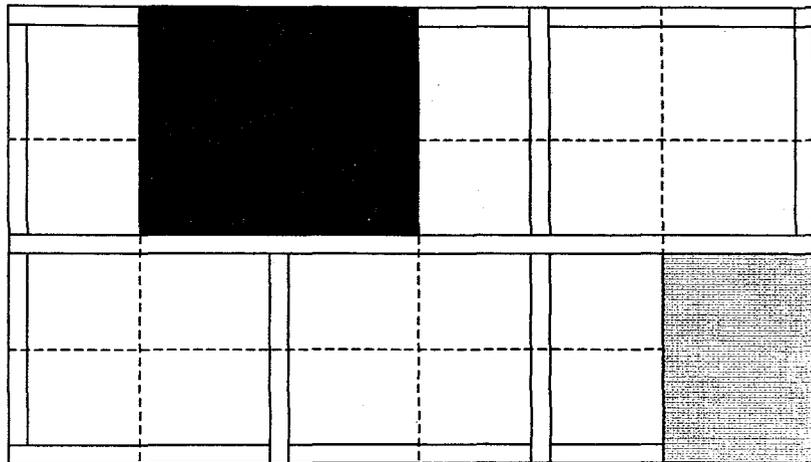
$$\text{Load on the Exterior beams} = 0.88429 \times 5 / 2 = 2.211 \text{ ton/m}$$

b) For the 7m by 7mspan of thickness 22 cm, the total weight exerted by the slab is:

$$W = 0.5126 + 2.323 \times 0.22 = 1.0237 \text{ ton/m}^2$$

$$\text{Therefore: Load on the Interior beams} = 1.0237 \times 7 = 7.166 \text{ ton/m}$$

$$\text{Load on the Exterior beams} = 1.0237 \times 7 / 2 = 3.583 \text{ ton/m}$$



Interior beams



Exterior beams

Fig. 6.2.1 - Showing the total slab area carried by the Interior and exterior beams

Considering equation 5.5.4.2, the values for the coefficient, C from design chart [14] are obtained and the weight of each floor, W is computed as the total weight of all elements located between two imaginary parallel planes passing through the midheight of columns above and below the floor considered. Having obtained the C coefficients and W for all sets

of structures, the lateral forces as well as the story and base shears were obtained using the programs in Appendix A and Appendix B for the 25 MPa and 45 MPa concrete respectively. Tables 6.2.1 –6.2.8 show the forces in the transverse and longitudinal directions respectively for 5m by 5m span 20, 15, 10 and 5 story buildings. Those for the other sets are given in the Appendices A and B.

Table 6.2.1 -Lateral forces in Transverse Direction for a 5m span 20 stories

<i>Floor Level From base</i>	<i>Height h (m)</i>	<i>Story weight ton</i>	<i>h*w ton-m</i>	<i>Lateral force ton</i>	<i>Story shear ton</i>
20	60	141.14	8468.64	17.76	17.76
19	57	141.14	8045.21	16.87	34.63
18	54	141.14	7621.78	15.98	50.61
17	51	141.14	7198.34	15.09	65.70
16	48	147.28	7069.34	14.82	80.52
15	45	147.28	6627.51	13.90	94.42
14	42	147.28	6185.68	12.97	107.39
13	39	156.89	6118.87	12.83	120.22
12	36	156.89	5648.18	11.84	132.06
11	33	156.89	5177.50	10.86	142.92
10	30	166.66	4999.92	10.48	153.40
9	27	166.66	4499.93	9.44	162.83
8	24	166.66	3999.94	8.39	171.22
7	21	166.66	3499.94	7.34	178.56
6	18	166.66	2999.95	6.29	184.85
5	15	178.08	2671.26	5.60	190.45
4	12	178.08	2137.01	4.48	194.93
3	9	178.08	1602.76	3.36	198.29
2	6	178.08	1068.50	2.24	200.53
1	3	178.08	534.25	1.12	201.65

Table 6.2.2 -Lateral forces in Transverse Direction for a 5m span 15 stories

<i>Floor Level</i> <i>From base</i>	<i>Height h</i> <i>(m)</i>	<i>Story weight</i> <i>ton</i>	<i>h*w</i> <i>ton-m</i>	<i>Lateral force</i> <i>ton</i>	<i>Story shear</i> <i>ton</i>
15	45	141.14	6351.48	19.10	19.10
14	42	141.14	5928.05	17.83	36.93
13	39	147.28	5743.84	17.27	54.20
12	36	147.28	5302.01	15.94	70.15
11	33	147.28	4860.17	14.62	84.76
10	30	156.89	4706.82	14.15	98.92
9	27	156.89	4236.14	12.74	111.66
8	24	156.89	3765.46	11.32	122.98
7	21	156.89	3294.77	9.91	132.89
6	18	156.89	2824.09	8.49	141.38
5	15	166.66	2499.96	7.52	148.90
4	12	166.66	1999.97	6.01	154.91
3	9	166.66	1499.98	4.51	159.42
2	6	166.66	999.98	3.01	162.43
1	3	166.66	499.99	1.50	163.93

Table 6.2.3 -Lateral forces in Transverse Direction for a 5m span 10 stories

10	30	147.28	4418.34	24.18	24.18
9	27	147.28	3976.51	21.76	45.94
8	24	147.28	3534.67	19.34	65.29
7	21	147.28	3092.84	16.93	82.21
6	18	147.28	2651.00	14.51	96.72
5	15	156.89	2353.41	12.88	109.60
4	12	156.89	1882.73	10.30	119.90
3	9	156.89	1412.05	7.73	127.63
2	6	156.89	941.36	5.15	132.78
1	3	156.89	470.68	2.58	135.36

Table 6.2.4 -Lateral forces in Transverse Direction for a 5m span 5 stories

5	15	147.28	2209.17	24.55	24.55
4	12	147.28	1767.34	19.64	44.18
3	9	147.28	1325.50	14.73	58.91
2	6	147.28	883.67	9.82	68.73
1	3	147.28	441.83	4.91	73.64

Table 6.2.5- Lateral forces in the Longitudinal Direction for a 5m span 20 stories

<i>Floor Level</i> <i>From base</i>	<i>Height h</i> <i>(m)</i>	<i>Story weight</i> <i>ton</i>	<i>h*w</i> <i>ton-m</i>	<i>Lateral force</i> <i>ton</i>	<i>Story shear</i> <i>ton</i>
20	60	141.14	8468.64	9.86	9.86
19	57	141.14	8045.21	9.37	19.24
18	54	141.14	7621.78	8.88	28.11
17	51	141.14	7198.34	8.39	36.50
16	48	147.28	7069.34	8.23	44.73
15	45	147.28	6627.51	7.72	52.45
14	42	147.28	6185.68	7.21	59.66
13	39	156.89	6118.87	7.13	66.79
12	36	156.89	5648.18	6.58	73.37
11	33	156.89	5177.50	6.03	79.40
10	30	166.66	4999.92	5.82	85.22
9	27	166.66	4499.93	5.24	90.46
8	24	166.66	3999.94	4.66	95.12
7	21	166.66	3499.94	4.08	99.20
6	18	166.66	2999.95	3.49	102.69
5	15	178.08	2671.26	3.11	105.81
4	12	178.08	2137.01	2.49	108.30
3	9	178.08	1602.76	1.87	110.16
2	6	178.08	1068.50	1.24	111.41
1	3	178.08	534.25	0.62	112.03

Table 6.2.6- Lateral forces in the Longitudinal Direction for a 5m span 15 stories

15	45	141.14	6351.48	10.91	10.91
14	42	141.14	5928.05	10.19	21.10
13	39	147.28	5743.84	9.87	30.97
12	36	147.28	5302.01	9.11	40.08
11	33	147.28	4860.17	8.35	48.44
10	30	156.89	4706.82	8.09	56.52
9	27	156.89	4236.14	7.28	63.80
8	24	156.89	3765.46	6.47	70.27
7	21	156.89	3294.77	5.66	75.94
6	18	156.89	2824.09	4.85	80.79
5	15	166.66	2499.96	4.30	85.08
4	12	166.66	1999.97	3.44	88.52
3	9	166.66	1499.98	2.58	91.10
2	6	166.66	999.98	1.72	92.82
1	3	166.66	499.99	0.86	93.68

Table 6.2.7- Lateral forces in the Longitudinal Direction for a 5m span 10 stories

10	30	147.28	4418.34	13.58	13.58
9	27	147.28	3976.51	12.23	25.81
8	24	147.28	3534.67	10.87	36.68
7	21	147.28	3092.84	9.51	46.19
6	18	147.28	2651.00	8.15	54.34
5	15	156.89	2353.41	7.24	61.57
4	12	156.89	1882.73	5.79	67.36
3	9	156.89	1412.05	4.34	71.70
2	6	156.89	941.36	2.89	74.60
1	3	156.89	470.68	1.45	76.04

Table 6.2.8- Lateral forces in the Longitudinal Direction for a 5m span 5 stories

5	15	147.28	2209.17	17.43	17.43
4	12	147.28	1767.34	13.94	31.37
3	9	147.28	1325.50	10.46	41.83
2	6	147.28	883.67	6.97	48.80
1	3	147.28	441.83	3.49	52.28

6.3 Structural Analysis of The Buildings

Three – dimensional analysis of the buildings were performed in both the transverse and longitudinal directions for the lateral load cases plus the gravity loads using SAP90 [15]. In the model, rigid diaphragms are assigned at each floor level (using Master Joints), and rigid-end offset were assigned at the ends of each horizontal member so that results were automatically obtained at the faces of each support. Appendix C gives the input and output files for 4 sets of structures. The output files comprehensively show all member forces from which the maximum Axial forces and Moments about both the X and Y-axes were obtained to design for reinforcement.

6.4 Design of Reinforcement

In designing for reinforcement, a program, shown in Appendix D, developed for biaxial bending of short rectangular columns with symmetrical arrangement of reinforcement [16] is used. The relevant input data required, as shown in Appendix D are: the concrete grade, steel strength, column breadth and depth, depths to steel, D1 and D2, axial load and Moments M_x and M_y . Table 6.4 below shows the output for a typical structure.

Table 6,4 - Design of the Reinforcement Cross Sections
5 story, 5m span and 25 Mpa concrete Strength

Column number	Column cross section (sq. mm)	Reinforcement cross section (sq. mm)
1	160000	640
2	160000	640
3	160000	640
4	160000	640
5	160000	640
6	250000	1000
7	250000	1000
8	160000	640
9	160000	640
10	160000	640
11	160000	640
12	160000	640
13	160000	640
14	160000	640
15	160000	640
16	160000	1036
17	160000	640
18	250000	1405
19	250000	1345
20	160000	1586
21	160000	925
22	160000	1559
23	160000	1566
24	160000	1395
25	160000	1395
26	160000	640
27	160000	640
28	160000	718
29	160000	640
30	250000	1000
31	250000	1000
32	160000	1326
33	160000	660
34	160000	1095
35	160000	1183
36	160000	1224
37	160000	640
38	160000	640
39	160000	640

Table 6.4 (continued)

Column number	Column cross section (sq. mm)	Reinforcement cross section (sq. mm)
40	160000	711
41	160000	640
42	250000	1000
43	250000	1000
44	160000	1232
45	160000	1232
46	160000	1663
47	160000	1163
48	160000	1294
49	160000	1294
50	160000	1294
51	160000	640
52	160000	906
53	160000	972
54	250000	1000
55	250000	1000
56	160000	1633
57	160000	1166
58	160000	1463
59	160000	1606
60	160000	1776

7. COST ANALYSIS

The material prices in the Turkish market for June 1998 is used to analyze the cost of columns used for the buildings under consideration. Prices are converted from Turkish Lira (TL) to more widely used currency, dollar (\$) using the immediate exchange rate. These prices are shown as below:

From TBS : 25 MPa \longrightarrow 12,100,000 TL/m³ = 47.12 \$/ m³
 45 MPa \longrightarrow 16,500,000 TL/m³ = 62.26 \$/ m³

From İÇDAŞ factory, steel of density $\rho = 7.85 \text{ ton/m}^3$, has the following range of prices:

<u>TL/Kg</u>	<u>\$/ Kg</u>
76,000	0.296
75,500	0.294
75,000	0.292
75,300	0.293

From these prices, 0.296 \$/ Kg is chosen and converted to 2323.6 \$/ m³ using the steel density above. Using the unit prices and the cross sections of concrete and steel obtained in the final design above, the cost of concrete, steel and the entire columns are computed for each set of building and are shown in tables 7.1 –7.4 and figures 7.1-7.8. From these tables savings made and the percentage savings with respect to the 25 MPa concrete for both span cases are computed and shown in tables 7.5 and 7.6 and figure 7.5.

Table 7.1. Cost of columns for the 5m Span Building using 25 Mpa concrete

Number of Stories	Cross Section (m2)		Volume (m3)		Cost of Concrete	Cost of Steel	Total cost of Columns
	Concrete	Steel	Concrete	Steel			
5	10,5	0,074978	31,5	0,224934	1484,28	522,656642	2006,936642
10	27,9	0,174944	83,7	0,524832	3943,944	1219,49964	5163,443635
15	51,2	0,305556	153,6	0,916668	7237,632	2129,96976	9367,601765
20	78,62	0,573768	235,86	1,721304	11113,7232	3999,62197	15113,34517

Table 7.2. Cost of columns for the 7m Span Building using 25 Mpa concrete

Number of Stories	Cross Section (m2)		Volume (m3)		Cost of Concrete	Cost of Steel	Total cost of Columns
	Concrete	Steel	Concrete	Steel			
5	22,9	0,10024	68,7	0,30072	3237,144	698,752992	3935,896992
10	55,5	0,297316	166,5	0,891948	7845,48	2072,53037	9918,010373
15	95,68	0,660244	287,04	1,980732	13525,3248	4602,42888	18127,75368
20	140,78	1,332618	422,34	3,997854	19900,6608	9289,41355	29190,07435

Table 7.3. Cost of columns for the 5m Span Building using 45 Mpa concrete

Number of Stories	Cross Section (m2)		Volume (m3)		Cost of Concrete	Cost of Steel	Total cost of Columns
	Concrete	Steel	Concrete	Steel			
5	6,1	0,101814	18,3	0,305442	1139,358	709,725031	1849,083031
10	16,6	0,211496	49,8	0,634488	3100,548	1474,29632	4574,844317
15	32,55	0,351216	97,65	1,053648	6079,689	2448,25649	8527,945493
20	53,54	0,50385	160,62	1,51155	10000,2012	3512,23758	13512,43878

Table 7.4. Cost of columns for the 7m Span Building using 45 Mpa concrete

Number of Stories	Cross Section (m2)		Volume (m3)		Cost of Concrete	Cost of Steel	Total cost of Columns
	Concrete	Steel	Concrete	Steel			
5	16,1	0,13256	48,3	0,39768	3007,158	924,049248	3931,207248
10	39	0,28223	117	0,84669	7284,42	1967,36888	9251,788884
15	67,66	0,530868	202,98	1,592604	12637,5348	3700,57465	16338,10945
20	102,8	0,952938	308,4	2,858814	19200,984	6642,74021	25843,72421

Table 7.5 Cost of Columns for 5m span for the two strength cases

Stories	fc= 25 Mpa	fc = 45 MPa	Savings	Percentage savings
5	2006,9366	1849,083	157,8536	7,865400432
10	5163,4436	4574,8443	588,5993	11,39935565
15	9367,602	8527,9455	839,6565	8,963409205
20	15113,34517	13512,439	1600,906	10,59266597

Table 7.6 Cost of Columns for 7m span for the two strength cases

Stories	fc= 25 Mpa	fc = 45 MPa	Savings	Percentage savings
5	3935,897	3931,207	4,69	0,119159622
10	9918,01	9251,7889	666,2211	6,717286028
15	18127,754	16338,109	1789,645	9,872403388
20	29190,07	25843,72	3346,35	11,46400129

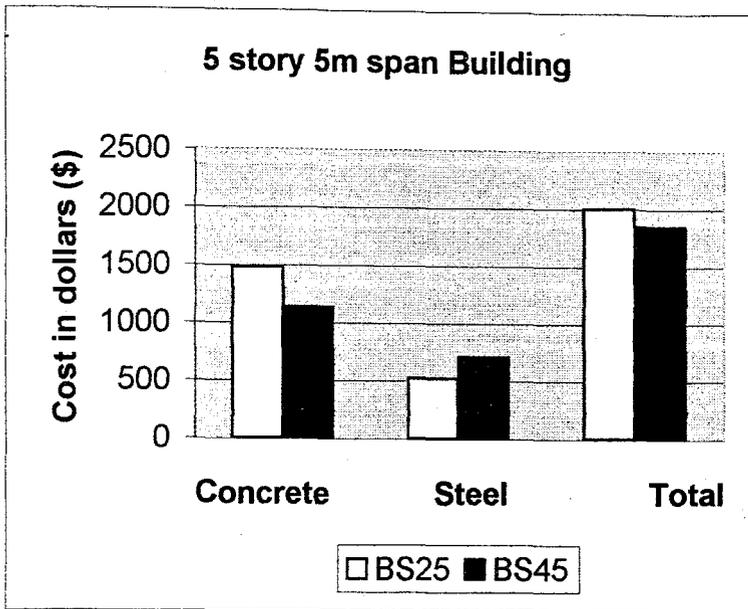


Figure 7.1 Cost for 5 story 5m span Building

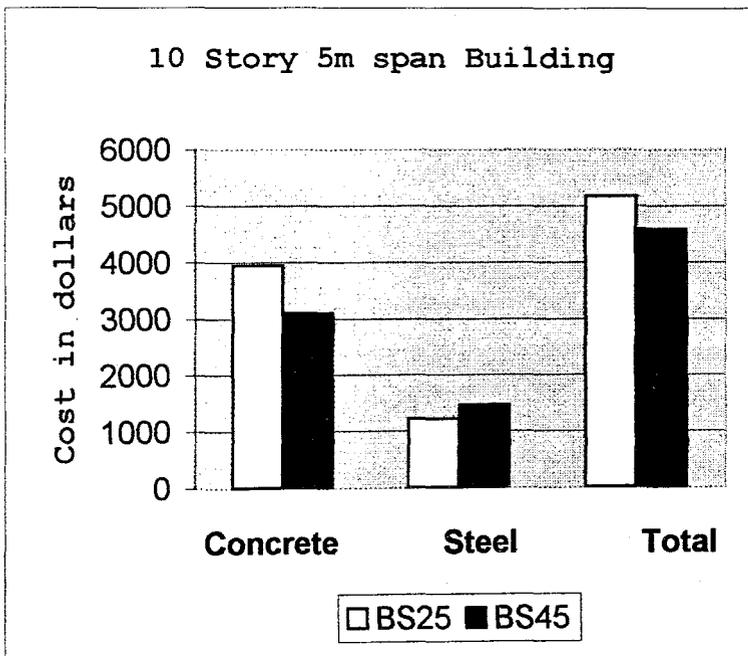


Figure 7.2 Cost for 10 story 5m span Building

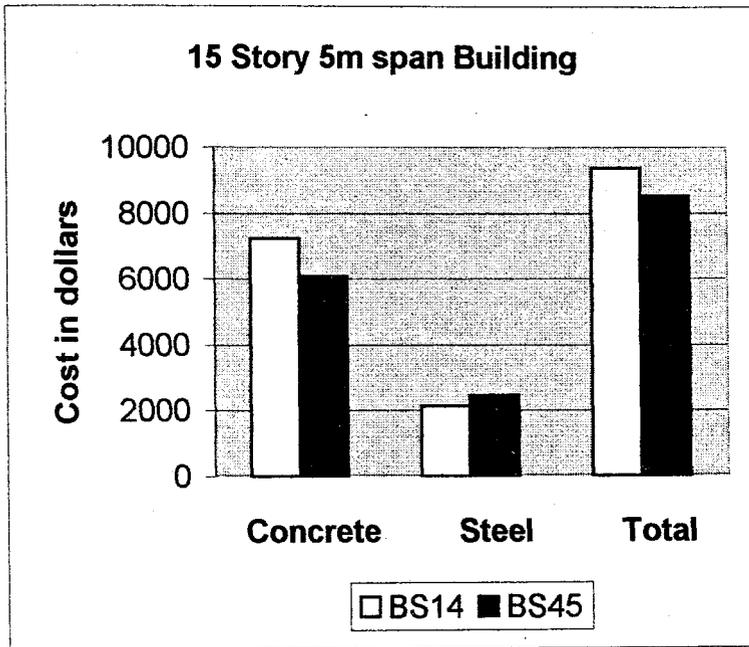


Figure 7.3 Cost for 15 story 5m span Building

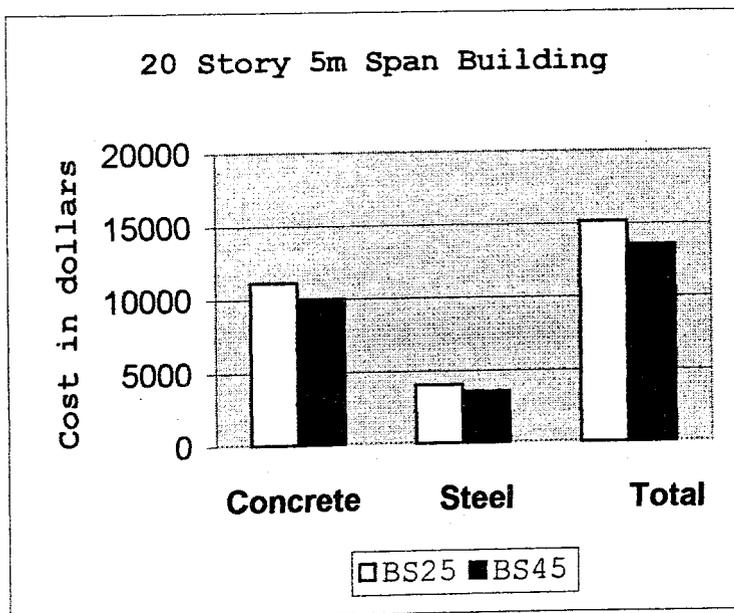


Figure 7.4 Cost for 20 story 5m span Building

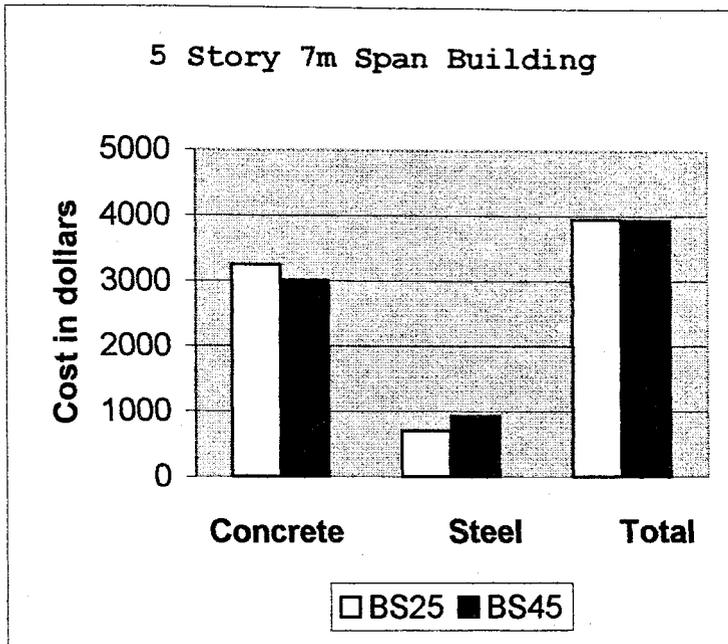


Figure 7.5 Cost for 5 story 7m span Building

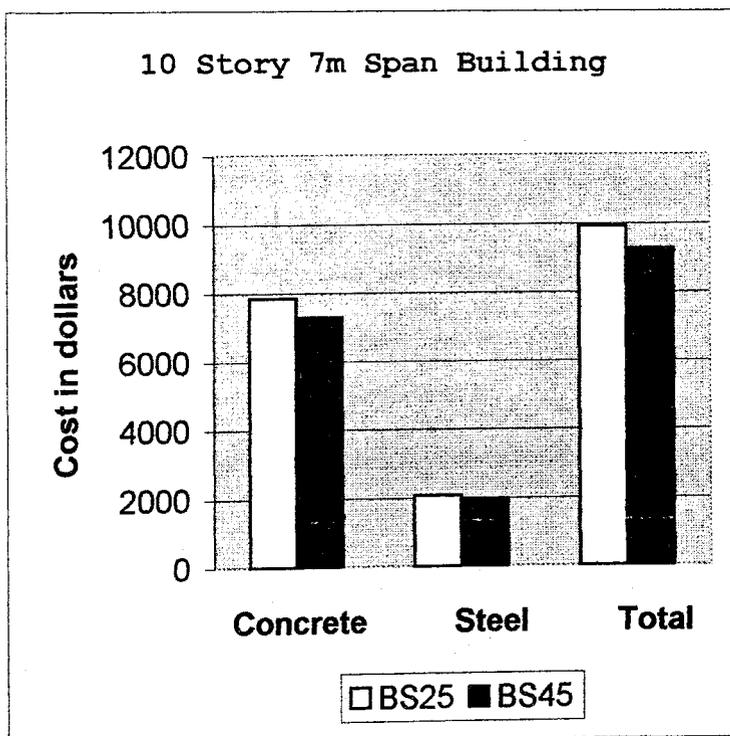


Figure 7.6 Cost for 10 story 7m span Building

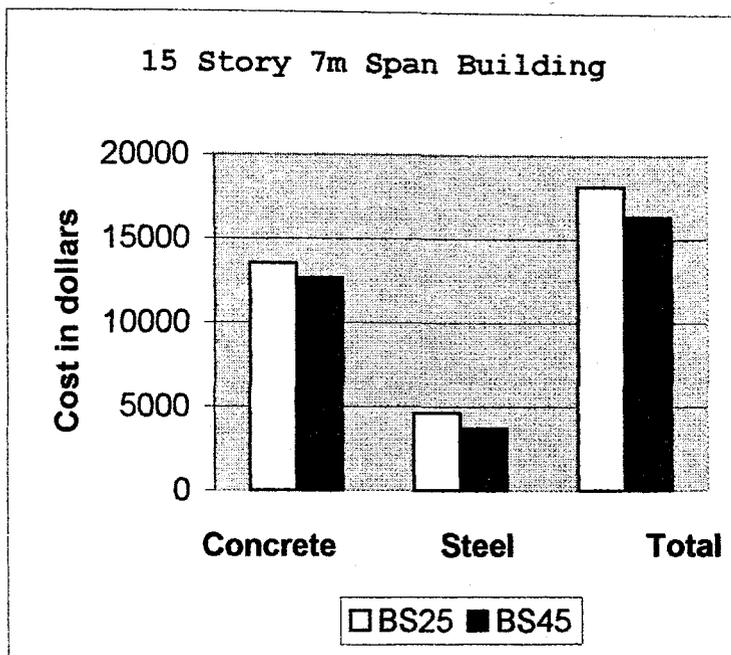


Figure 7.7 Cost for 15 story 7m span Building

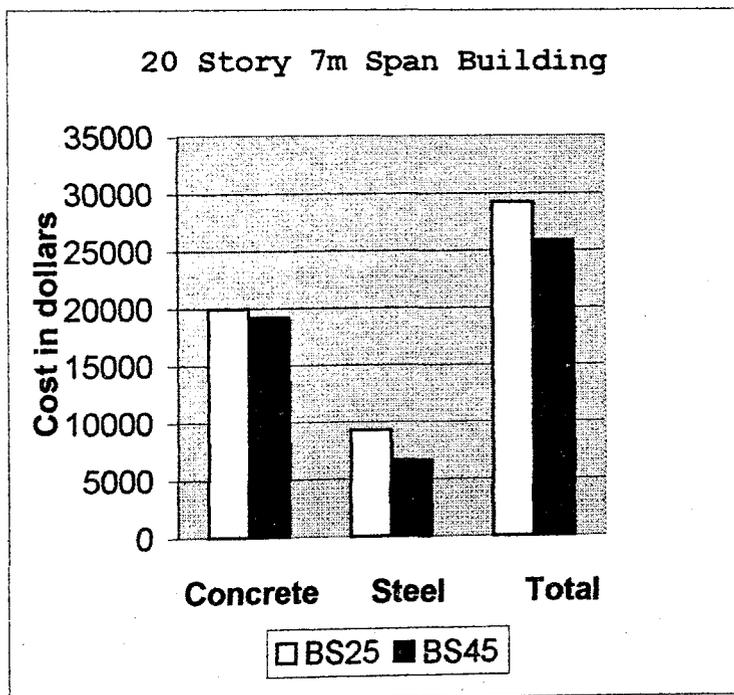


Figure 7.8 Cost for 20 story 7m span Building

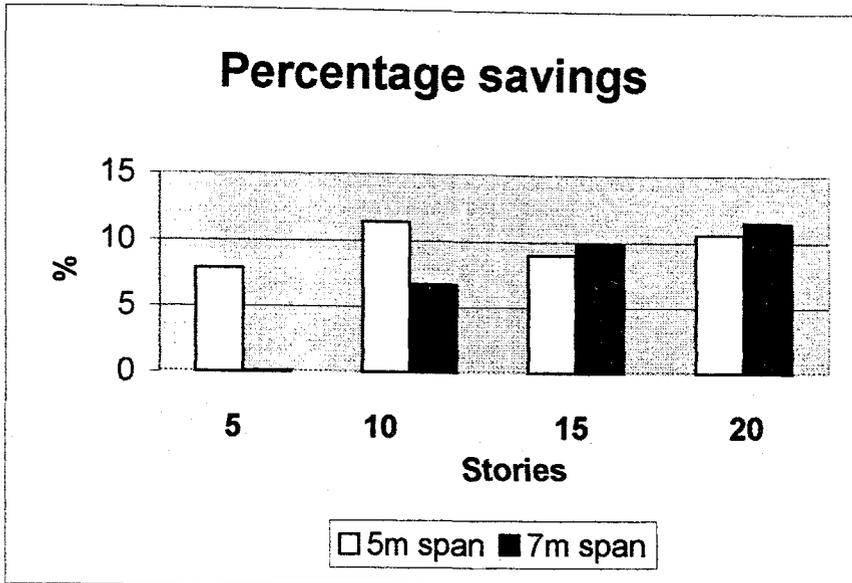


Figure 7.9 Percentage Savings for the 5m and 7m Spans

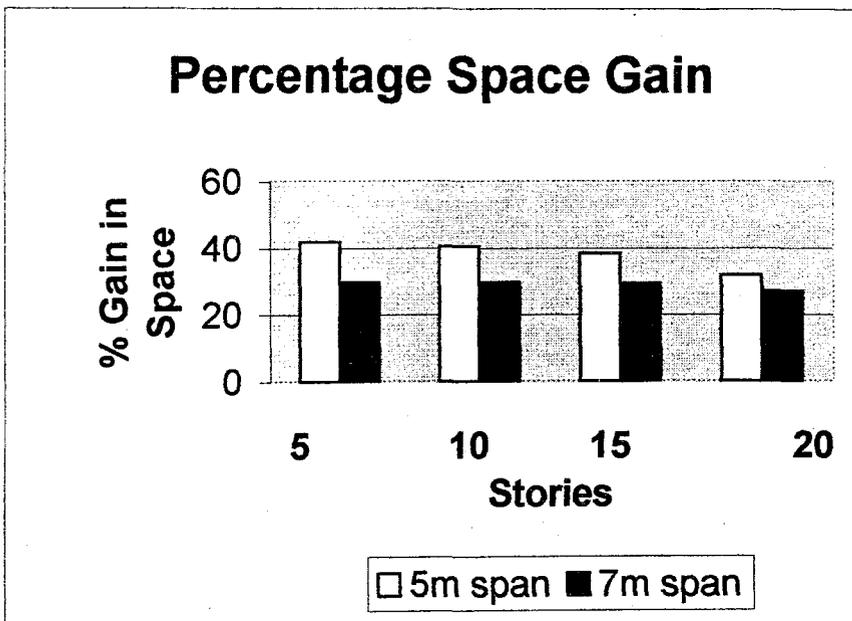


Figure 7.10 Percentage Space Gain for the 5m and 7m Spans

8. DISCUSSION OF THE RESULT

It is observed from figures 7.1 – 7.8, that the total cost of columns in all cases are higher for lower strength concrete than that of the higher strength. These results show significant change when the gross column cross section is changed, even by small amount. Thus with respect to the lower strength, substantial savings are observed when higher strength concrete is used.

Because of the irregular trend in the change in cross section for the BS45 case with respect to BS25, the amounts and costs of reinforcement used for BS45 are observed to be higher than that used for the lower strength, BS25, for some cases (figures: 7.1- 7.8), although the total cost is always higher for the higher strength.

Tables 7.1 through 7.6 and figure 7.9 indicate that the percentage saving observed in using high strength with respect to the lower strength increases with increase in the number of stories for both span cases, although some discrepancy is realized with the 10 story 5 span building which has higher saving than expected. This might have resulted from the column dimensions.

Figure 7.9 reveals that the percentage savings are generally more significant in the lower span, 5m, than in the case of 7m span. This occurrence is caused by the additional moment created by the longer span as compared to that of the shorter span, hence greater reduction in column cross section.

Other advantage in using high strength is the gain in rentable area. BS45 has shown remarkable amount of space gain with respect to lower strength, BS25. The benefit of space gain, unlike the percentage saving is more significant in higher stories (Figure 7.10)

Due to accumulated errors resulting from successive approximations, a few cases have violated the minimum reinforcing ratio, 0.1; these discrepancy, coupled with other factors, contribute to some deviations in the result from expected.

9. CONCLUSION

The information obtained and the result of the design analysis observed from the work provide the following conclusions:

- 1) With increase in concrete strength, there is a substantial decrease in the cost of column. In other words, saving is realized when a higher strength is used.
- 2) Keeping other factors constant, the percentage saving is always increasing with increase in stories.
- 3) Structures with smaller dimensions of floor panels realize more significant decrease in construction cost (savings). The reason is that smaller spans generate less moment and thus greater reduction in column cross section.
- 4) It is interesting to note that, when a high strength is used instead of a lower one, there is a significant gain in space. This is an important factor because, it increases rentable areas and therefore saves cost.
- 5) The percentage gain in space is observed to be decreasing with increase in story.

The achievement of the high strength stated above is the team effort of the whole concrete industry; the ready mixed concrete company has to produce it, to be handled by the contractor as specified by the designer and accepted by the code officials. Any reluctance on the part of any member of this team would strongly inhibit the achievement of this strength.

It is interesting to note that, besides its excellent properties, both chemically and mechanically, high strength concrete offers substantial benefits over lower strength concrete and even over steel as an axial load bearing construction material. This work although in a narrow scope, indicates that as well as reducing the cross sectional area of columns, using higher strength concrete provides cheaper columns.

Since actual relative costs for each item in reinforced concrete column construction vary in different localities, general principles for economy of column design are inapplicable.

It is recommended that for further study on the economy of high strength concrete, design is made using the same cross section over an entire floor and for all stories in multi story column; this is expected to save cost of form work which is approximately 30 percent of the entire construction cost, and erection time which consequently have a substantial reduction of the cost.

Appendix A

Lateral forces in Transverse Direction

5m square bay // 20 stories

<i>Floor Level from base</i>	<i>Height h (m)</i>	<i>Story weight tonnes</i>	<i>h*w ton-m</i>	<i>Lateral force t</i>	<i>Story shear t</i>
20	60	141.14	8468.64	17.76	17.76
19	57	141.14	8045.21	16.87	34.63
18	54	141.14	7621.78	15.98	50.61
17	51	141.14	7198.34	15.09	65.70
16	48	147.28	7069.34	14.82	80.52
15	45	147.28	6627.51	13.90	94.42
14	42	147.28	6185.68	12.97	107.39
13	39	156.89	6118.87	12.83	120.22
12	36	156.89	5648.18	11.84	132.06
11	33	156.89	5177.50	10.86	142.92
10	30	166.66	4999.92	10.48	153.40
9	27	166.66	4499.93	9.44	162.83
8	24	166.66	3999.94	8.39	171.22
7	21	166.66	3499.94	7.34	178.56
6	18	166.66	2999.95	6.29	184.85
5	15	178.08	2671.26	5.60	190.45
4	12	178.08	2137.01	4.48	194.93
3	9	178.08	1602.76	3.36	198.29
2	6	178.08	1068.50	2.24	200.53
1	3	178.08	534.25	1.12	201.65

5m square bay//15 stories

15	45	141.14	6351.48	19.10	19.10
14	42	141.14	5928.05	17.83	36.93
13	39	147.28	5743.84	17.27	54.20
12	36	147.28	5302.01	15.94	70.15
11	33	147.28	4860.17	14.62	84.76
10	30	156.89	4706.82	14.15	98.92
9	27	156.89	4236.14	12.74	111.66
8	24	156.89	3765.46	11.32	122.98
7	21	156.89	3294.77	9.91	132.89
6	18	156.89	2824.09	8.49	141.38
5	15	166.66	2499.96	7.52	148.90
4	12	166.66	1999.97	6.01	154.91
3	9	166.66	1499.98	4.51	159.42
2	6	166.66	999.98	3.01	162.43
1	3	166.66	499.99	1.50	163.93

5m square bay//10 stories

10	30	147.28	4418.34	24.18	24.18
9	27	147.28	3976.51	21.76	45.94
8	24	147.28	3534.67	19.34	65.29
7	21	147.28	3092.84	16.93	82.21
6	18	147.28	2651.00	14.51	96.72
5	15	156.89	2353.41	12.88	109.60
4	12	156.89	1882.73	10.30	119.90
3	9	156.89	1412.05	7.73	127.63
2	6	156.89	941.36	5.15	132.78
1	3	156.89	470.68	2.58	135.36

5m square bay//5 stories

5	15	147.28	2209.17	24.55	24.55
4	12	147.28	1767.34	19.64	44.18
3	9	147.28	1325.50	14.73	58.91
2	6	147.28	883.67	9.82	68.73
1	3	147.28	441.83	4.91	73.64

7m square bay//20 stories

<i>Floor Level from base</i>	<i>Height h (m)</i>	<i>Story weight tonnes</i>	<i>h*w ton-m</i>	<i>Lateral force t</i>	<i>Story shear t</i>
20	60	323.40	19404.00	42.50	42.50
19	57	323.40	18433.80	40.38	82.88
18	54	323.40	17463.60	38.25	121.13
17	51	323.40	16493.40	36.13	157.26
16	48	332.89	15978.72	35.00	192.26
15	45	332.89	14980.05	32.81	225.07
14	42	332.89	13981.38	30.62	255.69
13	39	346.40	13509.60	29.59	285.28
12	36	346.40	12470.40	27.31	312.60
11	33	346.40	11431.20	25.04	337.64
10	30	359.50	10784.94	23.62	361.26
9	27	359.50	9706.45	21.26	382.52
8	24	359.50	8627.95	18.90	401.42
7	21	359.50	7549.46	16.54	417.96
6	18	359.50	6470.96	14.17	432.13
5	15	374.27	5614.11	12.30	444.43
4	12	374.27	4491.29	9.84	454.27
3	9	374.27	3368.47	7.38	461.64
2	6	374.27	2245.64	4.92	466.56
1	3	374.27	1122.82	2.46	469.02

7m square bay//15 stories

15	45	323.40	14553.00	47.60	47.60
14	42	323.40	13582.80	44.42	92.02
13	39	332.89	12982.71	42.46	134.48
12	36	332.89	11984.04	39.20	173.68
11	33	332.89	10985.37	35.93	209.61
10	30	346.40	10392.00	33.99	243.60
9	27	346.40	9352.80	30.59	274.19
8	24	346.40	8313.60	27.19	301.38
7	21	346.40	7274.40	23.79	325.17
6	18	346.40	6235.20	20.39	345.56
5	15	359.50	5392.47	17.64	363.20
4	12	359.50	4313.98	14.11	377.31
3	9	359.50	3235.48	10.58	387.89
2	6	359.50	2156.99	7.05	394.94
1	3	359.50	1078.49	3.53	398.47

7m square bay//10 stories

10	30	332.89	9986.70	57.41	57.41
9	27	332.89	8988.03	51.67	109.08
8	24	332.89	7989.36	45.93	155.01
7	21	332.89	6990.69	40.19	195.20
6	18	332.89	5992.02	34.45	229.65
5	15	346.40	5196.00	29.87	259.52
4	12	346.40	4156.80	23.90	283.42
3	9	346.40	3117.60	17.92	301.34
2	6	346.40	2078.40	11.95	313.29
1	3	346.40	1039.20	5.97	319.27

7m square bay//5 stories

5	15	332.89	4993.35	55.48	55.48
4	12	332.89	3994.68	44.39	99.87
3	9	332.89	2996.01	33.29	133.16
2	6	332.89	1997.34	22.19	155.35
1	3	332.89	998.67	11.10	166.44

Lateral forces in Longitudinal Direction

5m square bay //20 stories

<i>Floor Level from base</i>	<i>Height h (m)</i>	<i>Story weight tonnes</i>	<i>h*w ton-m</i>	<i>Lateral force t</i>	<i>Story shear t</i>
20	60	141.14	8468.64	9.86	9.86
19	57	141.14	8045.21	9.37	19.24
18	54	141.14	7621.78	8.88	28.11
17	51	141.14	7198.34	8.39	36.50
16	48	147.28	7069.34	8.23	44.73
15	45	147.28	6627.51	7.72	52.45
14	42	147.28	6185.68	7.21	59.66
13	39	156.89	6118.87	7.13	66.79
12	36	156.89	5648.18	6.58	73.37
11	33	156.89	5177.50	6.03	79.40
10	30	166.66	4999.92	5.82	85.22
9	27	166.66	4499.93	5.24	90.46
8	24	166.66	3999.94	4.66	95.12
7	21	166.66	3499.94	4.08	99.20
6	18	166.66	2999.95	3.49	102.69
5	15	178.08	2671.26	3.11	105.81
4	12	178.08	2137.01	2.49	108.30
3	9	178.08	1602.76	1.87	110.16
2	6	178.08	1068.50	1.24	111.41
1	3	178.08	534.25	0.62	112.03

5m square bay //15 stories

15	45	141.14	6351.48	10.91	10.91
14	42	141.14	5928.05	10.19	21.10
13	39	147.28	5743.84	9.87	30.97
12	36	147.28	5302.01	9.11	40.08
11	33	147.28	4860.17	8.35	48.44
10	30	156.89	4706.82	8.09	56.52
9	27	156.89	4236.14	7.28	63.80
8	24	156.89	3765.46	6.47	70.27
7	21	156.89	3294.77	5.66	75.94
6	18	156.89	2824.09	4.85	80.79
5	15	166.66	2499.96	4.30	85.08
4	12	166.66	1999.97	3.44	88.52
3	9	166.66	1499.98	2.58	91.10
2	6	166.66	999.98	1.72	92.82
1	3	166.66	499.99	0.86	93.68

5m square bay //10 stories

10	30	147.28	4418.34	13.58	13.58
9	27	147.28	3976.51	12.23	25.81
8	24	147.28	3534.67	10.87	36.68
7	21	147.28	3092.84	9.51	46.19
6	18	147.28	2651.00	8.15	54.34
5	15	156.89	2353.41	7.24	61.57
4	12	156.89	1882.73	5.79	67.36
3	9	156.89	1412.05	4.34	71.70
2	6	156.89	941.36	2.89	74.60
1	3	156.89	470.68	1.45	76.04

5m square bay //5 stories

5	15	147.28	2209.17	17.43	17.43
4	12	147.28	1767.34	13.94	31.37
3	9	147.28	1325.50	10.46	41.83
2	6	147.28	883.67	6.97	48.80
1	3	147.28	441.83	3.49	52.28

7m square bay //20 stories

<i>Floor Level from base</i>	<i>Height h (m)</i>	<i>Story weight tonnes</i>	<i>h*w ton-m</i>	<i>Lateral force t</i>	<i>Story shear t</i>
20	60	323.40	19404.00	22.20	22.20
19	57	323.40	18433.80	21.09	43.30
18	54	323.40	17463.60	19.98	63.28
17	51	323.40	16493.40	18.87	82.15
16	48	332.89	15978.72	18.28	100.43
15	45	332.89	14980.05	17.14	117.57
14	42	332.89	13981.38	16.00	133.57
13	39	346.40	13509.60	15.46	149.03
12	36	346.40	12470.40	14.27	163.30
11	33	346.40	11431.20	13.08	176.38
10	30	359.50	10784.94	12.34	188.72
9	27	359.50	9706.45	11.11	199.83
8	24	359.50	8627.95	9.87	209.70
7	21	359.50	7549.46	8.64	218.34
6	18	359.50	6470.96	7.40	225.74
5	15	374.27	5614.11	6.42	232.16
4	12	374.27	4491.29	5.14	237.30
3	9	374.27	3368.47	3.85	241.16
2	6	374.27	2245.64	2.57	243.73
1	3	374.27	1122.82	1.28	245.01

7m square bay //15 stories

15	45	323.40	14553.00	24.73	24.73
14	42	323.40	13582.80	23.08	47.80
13	39	332.89	12982.71	22.06	69.86
12	36	332.89	11984.04	20.36	90.22
11	33	332.89	10985.37	18.66	108.89
10	30	346.40	10392.00	17.66	126.54
9	27	346.40	9352.80	15.89	142.43
8	24	346.40	8313.60	14.13	156.56
7	21	346.40	7274.40	12.36	168.92
6	18	346.40	6235.20	10.59	179.51
5	15	359.50	5392.47	9.16	188.67
4	12	359.50	4313.98	7.33	196.00
3	9	359.50	3235.48	5.50	201.50
2	6	359.50	2156.99	3.66	205.17
1	3	359.50	1078.49	1.83	207.00

7m square bay //10 stories

10	30	332.89	9986.70	30.54	30.54
9	27	332.89	8988.03	27.48	58.02
8	24	332.89	7989.36	24.43	82.45
7	21	332.89	6990.69	21.38	103.83
6	18	332.89	5992.02	18.32	122.16
5	15	346.40	5196.00	15.89	138.04
4	12	346.40	4156.80	12.71	150.76
3	9	346.40	3117.60	9.53	160.29
2	6	346.40	2078.40	6.36	166.64
1	3	346.40	1039.20	3.18	169.82

7m square bay //5stories

5	15	332.89	4993.35	39.39	39.39
4	12	332.89	3994.68	31.51	70.91
3	9	332.89	2996.01	23.64	94.54
2	6	332.89	1997.34	15.76	110.30
1	3	332.89	998.67	7.88	118.18

Appendix B

Lateral forces in Transverse Direction

5m span length // 20 Stories

<i>Floor Level from base</i>	<i>Height h (m)</i>	<i>Story weight tonnes</i>	<i>h*w ton-m</i>	<i>Lateral force t</i>	<i>Story shear t</i>
20	60	138.71	8322.72	17.25	17.25
19	57	138.71	7906.58	16.38	33.63
18	54	138.71	7490.45	15.52	49.15
17	51	138.71	7074.31	14.66	63.81
16	48	141.14	6774.91	14.04	77.85
15	45	141.14	6351.48	13.16	91.01
14	42	141.14	5928.05	12.28	103.29
13	39	147.28	5743.84	11.90	115.19
12	36	147.28	5302.01	10.99	126.18
11	33	147.28	4860.17	10.07	136.25
10	30	155.08	4652.52	9.64	145.89
9	27	155.08	4187.27	8.68	154.57
8	24	155.08	3722.02	7.71	162.28
7	21	155.08	3256.76	6.75	169.03
6	18	155.08	2791.51	5.78	174.81
5	15	164.57	2468.61	5.12	179.93
4	12	164.57	1974.89	4.09	184.02
3	9	164.57	1481.17	3.07	187.09
2	6	164.57	987.44	2.05	189.14
1	3	164.57	493.72	1.02	190.16

5m span length // 15 Stories

15	45	138.71	6242.04	18.61	18.61
14	42	138.71	5825.90	17.37	35.97
13	39	141.14	5504.62	16.41	52.38
12	36	141.14	5081.18	15.15	67.52
11	33	141.14	4657.75	13.88	81.41
10	30	147.28	4418.34	13.17	94.58
9	27	147.28	3976.51	11.85	106.43
8	24	147.28	3534.67	10.54	116.96
7	21	147.28	3092.84	9.22	126.18
6	18	147.28	2651.00	7.90	134.09
5	15	155.08	2326.26	6.93	141.02
4	12	155.08	1861.01	5.55	146.57
3	9	155.08	1395.76	4.16	150.73
2	6	155.08	930.50	2.77	153.50
1	3	155.08	465.25	1.39	154.89

5m span length // 10 Stories

10	30	141.14	4234.32	23.06	23.06
9	27	141.14	3810.89	20.76	43.82
8	24	141.14	3387.46	18.45	62.27
7	21	141.14	2964.02	16.14	78.41
6	18	141.14	2540.59	13.84	92.25
5	15	147.28	2209.17	12.03	104.28
4	12	147.28	1767.34	9.63	113.91
3	9	147.28	1325.50	7.22	121.13
2	6	147.28	883.67	4.81	125.94
1	3	147.28	441.83	2.41	128.35

5m span length // 5 Stories

5	15	141.14	2117.16	23.52	23.52
4	12	141.14	1693.73	18.82	42.34
3	9	141.14	1270.30	14.11	56.46
2	6	141.14	846.86	9.41	65.87
1	3	141.14	423.43	4.70	70.57

7m span length // 20 Stories

<i>Floor Level</i> <i>from base</i>	<i>Height h</i> <i>(m)</i>	<i>Story weight w</i> <i>tonnes</i>	<i>h*w</i> <i>ton-m</i>	<i>Lateral force</i> <i>t</i>	<i>Story shear</i> <i>t</i>
20	60	315.59	18935.64	41.28	41.28
19	57	315.59	17988.86	39.22	80.49
18	54	315.59	17042.08	37.15	117.65
17	51	315.59	16095.29	35.09	152.73
16	48	323.40	15523.20	33.84	186.57
15	45	323.40	14553.00	31.73	218.30
14	42	323.40	13582.80	29.61	247.91
13	39	332.89	12982.71	28.30	276.21
12	36	332.89	11984.04	26.12	302.34
11	33	332.89	10985.37	23.95	326.28
10	30	344.03	10320.90	22.50	348.78
9	27	344.03	9288.81	20.25	369.03
8	24	344.03	8256.72	18.00	387.03
7	21	344.03	7224.63	15.75	402.78
6	18	344.03	6192.54	13.50	416.28
5	15	356.85	5352.75	11.67	427.95
4	12	356.85	4282.20	9.34	437.29
3	9	356.85	3211.65	7.00	444.29
2	6	356.85	2141.10	4.67	448.95
1	3	356.85	1070.55	2.33	451.29

7m span length // 15 Stories

15	45	315.59	14201.73	46.28	46.28
14	42	315.59	13254.95	43.19	89.47
13	39	323.40	12612.60	41.10	130.58
12	36	323.40	11642.40	37.94	168.52
11	33	323.40	10672.20	34.78	203.29
10	30	332.89	9986.70	32.54	235.84
9	27	332.89	8988.03	29.29	265.13
8	24	332.89	7989.36	26.04	291.16
7	21	332.89	6990.69	22.78	313.94
6	18	332.89	5992.02	19.53	333.47
5	15	344.03	5160.45	16.82	350.29
4	12	344.03	4128.36	13.45	363.74
3	9	344.03	3096.27	10.09	373.83
2	6	344.03	2064.18	6.73	380.56
1	3	344.03	1032.09	3.36	383.92

7m span length // 10 Stories

10	30	323.40	9702.00	55.64	55.64
9	27	323.40	8731.80	50.07	105.71
8	24	323.40	7761.60	44.51	150.22
7	21	323.40	6791.40	38.95	189.17
6	18	323.40	5821.20	33.38	222.55
5	15	332.89	4993.35	28.64	251.19
4	12	332.89	3994.68	22.91	274.09
3	9	332.89	2996.01	17.18	291.28
2	6	332.89	1997.34	11.45	302.73
1	3	332.89	998.67	5.73	308.46

7m span length // 5Stories

5	15	323.40	4851.00	53.90	53.90
4	12	323.40	3880.80	43.12	97.02
3	9	323.40	2910.60	32.34	129.36
2	6	323.40	1940.40	21.56	150.92
1	3	323.40	970.20	10.78	161.70

Lateral forces in Longitudinal Direction

5m span length // 20 Stories

<i>Floor Level from base</i>	<i>Height h (m)</i>	<i>Story weight w tonnes</i>	<i>h*w ton-m</i>	<i>Lateral force t</i>	<i>Story shear t</i>
20	60	138.71	8322.72	9.58	9.58
19	57	138.71	7906.58	9.10	18.68
18	54	138.71	7490.45	8.62	27.31
17	51	138.71	7074.31	8.14	35.45
16	48	141.14	6774.91	7.80	43.25
15	45	141.14	6351.48	7.31	50.56
14	42	141.14	5928.05	6.82	57.38
13	39	147.28	5743.84	6.61	64.00
12	36	147.28	5302.01	6.10	70.10
11	33	147.28	4860.17	5.59	75.70
10	30	155.08	4652.52	5.36	81.05
9	27	155.08	4187.27	4.82	85.87
8	24	155.08	3722.02	4.28	90.16
7	21	155.08	3256.76	3.75	93.91
6	18	155.08	2791.51	3.21	97.12
5	15	164.57	2468.61	2.84	99.96
4	12	164.57	1974.89	2.27	102.23
3	9	164.57	1481.17	1.71	103.94
2	6	164.57	987.44	1.14	105.08
1	3	164.57	493.72	0.57	105.64

5m span length // 15 Stories

15	45	138.71	6242.04	10.63	10.63
14	42	138.71	5825.90	9.92	20.55
13	39	141.14	5504.62	9.38	29.93
12	36	141.14	5081.18	8.65	38.58
11	33	141.14	4657.75	7.93	46.52
10	30	147.28	4418.34	7.53	54.04
9	27	147.28	3976.51	6.77	60.82
8	24	147.28	3534.67	6.02	66.84
7	21	147.28	3092.84	5.27	72.10
6	18	147.28	2651.00	4.52	76.62
5	15	155.08	2326.26	3.96	80.58
4	12	155.08	1861.01	3.17	83.75
3	9	155.08	1395.76	2.38	86.13
2	6	155.08	930.50	1.58	87.71
1	3	155.08	465.25	0.79	88.51

5m span length // 10 Stories

10	30	141.14	4234.32	12.96	12.96
9	27	141.14	3810.89	11.66	24.62
8	24	141.14	3387.46	10.37	34.98
7	21	141.14	2964.02	9.07	44.05
6	18	141.14	2540.59	7.77	51.83
5	15	147.28	2209.17	6.76	58.59
4	12	147.28	1767.34	5.41	63.99
3	9	147.28	1325.50	4.06	68.05
2	6	147.28	883.67	2.70	70.75
1	3	147.28	441.83	1.35	72.11

5m span length // 5 Stories

5	15	141.14	2117.16	16.70	16.70
4	12	141.14	1693.73	13.36	30.06
3	9	141.14	1270.30	10.02	40.08
2	6	141.14	846.86	6.68	46.77
1	3	141.14	423.43	3.34	50.11

7m span length // 20 Stories

<i>Floor Level from base</i>	<i>Height h (m)</i>	<i>Story weight w tonnes</i>	<i>h*w ton-m</i>	<i>Lateral force t</i>	<i>Story shear t</i>
20	60	315.59	18935.64	21.56	21.56
19	57	315.59	17988.86	20.49	42.05
18	54	315.59	17042.08	19.41	61.46
17	51	315.59	16095.29	18.33	79.79
16	48	323.40	15523.20	17.68	97.46
15	45	323.40	14553.00	16.57	114.04
14	42	323.40	13582.80	15.47	129.50
13	39	332.89	12982.71	14.78	144.29
12	36	332.89	11984.04	13.65	157.94
11	33	332.89	10985.37	12.51	170.45
10	30	344.03	10320.90	11.75	182.20
9	27	344.03	9288.81	10.58	192.78
8	24	344.03	8256.72	9.40	202.18
7	21	344.03	7224.63	8.23	210.41
6	18	344.03	6192.54	7.05	217.46
5	15	356.85	5352.75	6.10	223.56
4	12	356.85	4282.20	4.88	228.43
3	9	356.85	3211.65	3.66	232.09
2	6	356.85	2141.10	2.44	234.53
1	3	356.85	1070.55	1.22	235.75

7m span length // 15 Stories

15	45	315.59	14201.73	24.04	24.04
14	42	315.59	13254.95	22.44	46.48
13	39	323.40	12612.60	21.35	67.83
12	36	323.40	11642.40	19.71	87.54
11	33	323.40	10672.20	18.07	105.61
10	30	332.89	9986.70	16.91	122.51
9	27	332.89	8988.03	15.22	137.73
8	24	332.89	7989.36	13.52	151.25
7	21	332.89	6990.69	11.83	163.09
6	18	332.89	5992.02	10.14	173.23
5	15	344.03	5160.45	8.74	181.97
4	12	344.03	4128.36	6.99	188.96
3	9	344.03	3096.27	5.24	194.20
2	6	344.03	2064.18	3.49	197.69
1	3	344.03	1032.09	1.75	199.44

7m span length // 10 Stories

10	30	323.40	9702.00	29.59	29.59
9	27	323.40	8731.80	26.64	56.23
8	24	323.40	7761.60	23.68	79.91
7	21	323.40	6791.40	20.72	100.62
6	18	323.40	5821.20	17.76	118.38
5	15	332.89	4993.35	15.23	133.61
4	12	332.89	3994.68	12.19	145.79
3	9	332.89	2996.01	9.14	154.93
2	6	332.89	1997.34	6.09	161.03
1	3	332.89	998.67	3.05	164.07

7m span length // 5 Stories

5	15	323.40	4851.00	38.27	38.27
4	12	323.40	3880.80	30.62	68.88
3	9	323.40	2910.60	22.96	91.85
2	6	323.40	1940.40	15.31	107.15
1	3	323.40	970.20	7.65	114.81

Appendix C

20 storey building - Span length 5*5// Fc=25MPa

Units :KN-m

SYSTEM

L=2

JOINTS

1	X=0	Y=0	Z=0	
4	X=15	Y=0	Z=0	
241	X=0	Y=0	Z=60	
244	X=15	Y=0	Z=60	Q=1,4,241,244,1,12
5	X=0	Y=5	Z=0	
8	X=15	Y=5	Z=0	
245	X=0	Y=5	Z=60	
248	X=15	Y=5	Z=60	Q=5,8,245,248,1,12
9	X=0	Y=10	Z=0	
12	X=15	Y=10	Z=0	
249	X=0	Y=10	Z=60	
252	X=15	Y=10	Z=60	Q=9,12,249,252,1,12
501	X=7.5	Y=5	Z=3	
520	X=7.5	Y=5	Z=60	G=501,520,1

RESTRAINTS

13,252,1	R=1,1,0,0,0,1	
1,12,1	R=1,1,1,1,1,1	:FIX BASE
501,520,1	R=0,0,1,1,1,0	

FRAME

NM=8 NL=2

C total of 8 member section properties & 2 span loading configurations

1	SH=R	T=0.9,0.9	E=2.4038e7
2	SH=R	T=0.8,0.8	
3	SH=R	T=0.7,0.7	
4	SH=R	T=0.6,0.6	
5	SH=R	T=0.5,0.5	
6	SH=R	T=0.4,0.4	
7	SH=R	T=0.3,0.3	
8	SH=R	T=0.5,0.25	

C Below is the span loading(dead +live)data. Values in t/m length of beam

1	WG=0,0,-4.421
2	WG=0,0,-2.221

C Exterior columns for levels 1-5

1,1,13	G=4,12,12,12,1,1	M=3	LP=3,0	MS=0,501
2,2,14	G=4,12,12,12,1,1	M=3	LP=3,0	MS=0,501
3,3,15	G=4,12,12,12,1,1	M=3	LP=3,0	MS=0,501
4,4,16	G=4,12,12,12,1,1	M=3	LP=3,0	MS=0,501
5,5,17	G=4,12,12,12,1,1	M=3	LP=3,0	MS=0,501
8,8,20	G=4,12,12,12,1,1	M=3	LP=3,0	MS=0,501
9,9,21	G=4,12,12,12,1,1	M=3	LP=3,0	MS=0,501
10,10,22	G=4,12,12,12,1,1	M=3	LP=3,0	MS=0,501
11,11,23	G=4,12,12,12,1,1	M=3	LP=3,0	MS=0,501
12,12,24	G=4,12,12,12,1,1	M=3	LP=3,0	MS=0,501

C Interior columns -levels 1-5

6,6,18	G=4,12,12,12,1,1	M=1	LP=3,0	MS=0,501
7,7,19	G=4,12,12,12,1,1	M=1	LP=3,0	MS=0,501

C Exterior columns for levels 6-10

61,61,73	G=4,12,12,12,1,1	M=4	LP=3,0	MS=505,506
62,62,74	G=4,12,12,12,1,1	M=4	LP=3,0	MS=505,506
63,63,75	G=4,12,12,12,1,1	M=4	LP=3,0	MS=505,506
64,64,76	G=4,12,12,12,1,1	M=4	LP=3,0	MS=505,506
65,65,77	G=4,12,12,12,1,1	M=4	LP=3,0	MS=505,506

68,68,80	G=4,12,12,12,1,1	M=4	LP=3,0	MS=505,506
69,69,81	G=4,12,12,12,1,1	M=4	LP=3,0	MS=505,506
70,70,82	G=4,12,12,12,1,1	M=4	LP=3,0	MS=505,506
71,71,83	G=4,12,12,12,1,1	M=4	LP=3,0	MS=505,506
72,72,84	G=4,12,12,12,1,1	M=4	LP=3,0	MS=505,506
C Interior columns for levels 6-10				
66,66,78	G=4,12,12,12,1,1	M=2	LP=3,0	MS=505,506
67,67,79	G=4,12,12,12,1,1	M=2	LP=3,0	MS=505,506
C Exterior columns for levels 11-13				
121,121,133	G=2,12,12,12,1,1	M=5	LP=3,0	MS=510,511
122,122,134	G=2,12,12,12,1,1	M=5	LP=3,0	MS=510,511
123,123,135	G=2,12,12,12,1,1	M=5	LP=3,0	MS=510,511
124,124,136	G=2,12,12,12,1,1	M=5	LP=3,0	MS=510,511
125,125,137	G=2,12,12,12,1,1	M=5	LP=3,0	MS=510,511
128,128,140	G=2,12,12,12,1,1	M=5	LP=3,0	MS=510,511
129,129,141	G=2,12,12,12,1,1	M=5	LP=3,0	MS=510,511
130,130,142	G=2,12,12,12,1,1	M=5	LP=3,0	MS=510,511
131,131,143	G=2,12,12,12,1,1	M=5	LP=3,0	MS=510,511
132,132,144	G=2,12,12,12,1,1	M=5	LP=3,0	MS=510,511
C Interior columns levels 11-13				
126,126,138	G=2,12,12,12,1,1	M=3	LP=3,0	MS=510,511
127,127,139	G=2,12,12,12,1,1	M=3	LP=3,0	MS=510,511
C Exterior columns levels 14-16				
157,157,169	G=2,12,12,12,1,1	M=6	LP=3,0	MS=513,514
158,158,170	G=2,12,12,12,1,1	M=6	LP=3,0	MS=513,514
159,159,171	G=2,12,12,12,1,1	M=6	LP=3,0	MS=513,514
160,160,172	G=2,12,12,12,1,1	M=6	LP=3,0	MS=513,514
161,161,173	G=2,12,12,12,1,1	M=6	LP=3,0	MS=513,514
164,164,176	G=2,12,12,12,1,1	M=6	LP=3,0	MS=513,514
165,165,177	G=2,12,12,12,1,1	M=6	LP=3,0	MS=513,514
166,166,178	G=2,12,12,12,1,1	M=6	LP=3,0	MS=513,514
167,167,179	G=2,12,12,12,1,1	M=6	LP=3,0	MS=513,514
168,168,180	G=2,12,12,12,1,1	M=6	LP=3,0	MS=513,514
C Interior columns levels 14-16				
162,162,174	G=2,12,12,12,1,1	M=5	LP=3,0	MS=513,514
163,163,175	G=2,12,12,12,1,1	M=5	LP=3,0	MS=513,514
C Exterior columns levels 17-20				
193,193,205	G=3,12,12,12,1,1	M=7	LP=3,0	MS=516,517
194,194,206	G=3,12,12,12,1,1	M=7	LP=3,0	MS=516,517
195,195,207	G=3,12,12,12,1,1	M=7	LP=3,0	MS=516,517
196,196,208	G=3,12,12,12,1,1	M=7	LP=3,0	MS=516,517
197,197,209	G=3,12,12,12,1,1	M=7	LP=3,0	MS=516,517
200,200,212	G=3,12,12,12,1,1	M=7	LP=3,0	MS=516,517
201,201,213	G=3,12,12,12,1,1	M=7	LP=3,0	MS=516,517
202,202,214	G=3,12,12,12,1,1	M=7	LP=3,0	MS=516,517
203,203,215	G=3,12,12,12,1,1	M=7	LP=3,0	MS=516,517
204,204,216	G=3,12,12,12,1,1	M=7	LP=3,0	MS=516,517
C Interior columns levels 17-20				
198,198,210	G=3,12,12,12,1,1	M=6	LP=3,0	MS=516,517
199,199,211	G=3,12,12,12,1,1	M=6	LP=3,0	MS=516,517
C BEAMS				
C Exterior beams				
241,13,14	G=19,1,12,12,1,1	M=8	LP=2,0	NSL=2 MS=501,501
261,14,15	G=19,1,12,12,1,1	M=8	LP=2,0	NSL=2 MS=501,501
281,15,16	G=19,1,12,12,1,1	M=8	LP=2,0	NSL=2 MS=501,501
361,21,22	G=19,1,12,12,1,1	M=8	LP=2,0	NSL=2 MS=501,501
381,22,23	G=19,1,12,12,1,1	M=8	LP=2,0	NSL=2 MS=501,501
401,23,24	G=19,1,12,12,1,1	M=8	LP=2,0	NSL=2 MS=501,501
421,13,17	G=19,1,12,12,1,1	M=8	LP=3,0	NSL=2 MS=501,501
441,17,21	G=19,1,12,12,1,1	M=8	LP=3,0	NSL=2 MS=501,501

541,16,20	G=19,1,12,12,1,1	M=8	LP=3,0	NSL=2	MS=501,501
561,20,24	G=19,1,12,12,1,1	M=8	LP=3,0	NSL=2	MS=501,501
C Interior beams					
461,14,18	G=19,1,12,12,1,1	M=8	LP=3,0	NSL=1	MS=501,501
481,18,22	G=19,1,12,12,1,1	M=8	LP=3,0	NSL=1	MS=501,501
501,15,19	G=19,1,12,12,1,1	M=8	LP=3,0	NSL=1	MS=501,501
521,19,23	G=19,1,12,12,1,1	M=8	LP=3,0	NSL=1	MS=501,501
301,17,18	G=19,1,12,12,1,1	M=8	LP=2,0	NSL=1	MS=501,501
321,18,19	G=19,1,12,12,1,1	M=8	LP=2,0	NSL=1	MS=501,501
341,19,20	G=19,1,12,12,1,1	M=8	LP=2,0	NSL=1	MS=501,501

LOADS

C Lateral Joint Loads(Earthquake + Wind) in the transverse direction

C ie parallel to the y-axis

501	F=0,1.12,0	L=2
502	F=0,2.24,0	
503	F=0,3.36,0	
504	F=0,4.48,0	
505	F=0,5.60,0	
506	F=0,6.29,0	
507	F=0,7.34,0	
508	F=0,8.39,0	
509	F=0,9.44,0	
510	F=0,10.48,0	
511	F=0,10.86,0	
512	F=0,11.84,0	
513	F=0,12.83,0	
514	F=0,12.97,0	
515	F=0,13.90,0	
516	F=0,14.82,0	
517	F=0,15.09,0	
518	F=0,15.98,0	
519	F=0,16.87,0	
520	F=0,17.76,0	

C 30% of Lateral Join Loads in the transverse direction

C ie parallel to the x-axis

501	F=0.62,0,0
502	F=1.24,0,0
503	F=1.87,0,0
504	F=2.49,0,0
505	F=3.11,0,0
506	F=3.49,0,0
507	F=4.08,0,0
508	F=4.66,0,0
509	F=5.24,0,0
510	F=5.82,0,0
511	F=6.03,0,0
512	F=6.58,0,0
513	F=7.13,0,0
514	F=7.21,0,0
515	F=7.72,0,0
516	F=8.23,0,0
517	F=8.39,0,0
518	F=8.88,0,0
519	F=9.37,0,0
520	F=9.86,0,0

COMBO

1 C=9.81,9.81

SELECT

NT=1 ID=1,252 SW=1

NT=2 ID=1,252 SW=1

NT=5 ID=1,240 SW=1

15 storey building - Span length 5*5 // Fc=25MPa

Unit :KN-m

SYSTEM

L=2

JOINTS

1	X=0	Y=0	Z=0	
4	X=15	Y=0	Z=0	
181	X=0	Y=0	Z=45	
184	X=15	Y=0	Z=45	Q=1,4,181,184,1,12
5	X=0	Y=5	Z=0	
8	X=15	Y=5	Z=0	
185	X=0	Y=5	Z=45	
188	X=15	Y=5	Z=45	Q=5,8,185,188,1,12
9	X=0	Y=10	Z=0	
12	X=15	Y=10	Z=0	
189	X=0	Y=10	Z=45	
192	X=15	Y=10	Z=45	Q=9,12,189,192,1,12
501	X=7.5	Y=5	Z=3	
515	X=7.5	Y=5	Z=45	G=501,515,1

RESTRAINTS

13,252,1	R=1,1,0,0,0,1	
1,12,1	R=1,1,1,1,1,1	: FIX BASE
501,515,1	R=0,0,1,1,1,0	

FRAME

NM=7 NL=2

C total of 8 member section properties & 2 span loading configurations

1	SH=R	T=0.8,0.8	E=2.4038e7
2	SH=R	T=0.7,0.7	
3	SH=R	T=0.6,0.6	
4	SH=R	T=0.5,0.5	
5	SH=R	T=0.4,0.4	
6	SH=R	T=0.3,0.3	
7	SH=R	T=0.5,0.25	

C Below is the span loading(dead +live)data. Values in t/m length of beam

1	WG=0,0,-4.421
2	WG=0,0,-2.211

C Exterior columns for levels 1-5

1,1,13	G=4,12,12,12,1,1	M=3	LP=3,0	MS=0,501
2,2,14	G=4,12,12,12,1,1	M=3	LP=3,0	MS=0,501
3,3,15	G=4,12,12,12,1,1	M=3	LP=3,0	MS=0,501
4,4,16	G=4,12,12,12,1,1	M=3	LP=3,0	MS=0,501
5,5,17	G=4,12,12,12,1,1	M=3	LP=3,0	MS=0,501
8,8,20	G=4,12,12,12,1,1	M=3	LP=3,0	MS=0,501
9,9,21	G=4,12,12,12,1,1	M=3	LP=3,0	MS=0,501
10,10,22	G=4,12,12,12,1,1	M=3	LP=3,0	MS=0,501
11,11,23	G=4,12,12,12,1,1	M=3	LP=3,0	MS=0,501
12,12,24	G=4,12,12,12,1,1	M=3	LP=3,0	MS=0,501

C Interior columns -levels 1-5

6,6,18	G=4,12,12,12,1,1	M=1	LP=3,0	MS=0,501
7,7,19	G=4,12,12,12,1,1	M=1	LP=3,0	MS=0,501

C Exterior columns for levels 6-10

61,61,73	G=4,12,12,12,1,1	M=4	LP=3,0	MS=505,506
62,62,74	G=4,12,12,12,1,1	M=4	LP=3,0	MS=505,506
63,63,75	G=4,12,12,12,1,1	M=4	LP=3,0	MS=505,506
64,64,76	G=4,12,12,12,1,1	M=4	LP=3,0	MS=505,506
65,65,77	G=4,12,12,12,1,1	M=4	LP=3,0	MS=505,506
68,68,80	G=4,12,12,12,1,1	M=4	LP=3,0	MS=505,506

69,69,81	G=4,12,12,12,1,1	M=4	LP=3,0	MS=505,506
70,70,82	G=4,12,12,12,1,1	M=4	LP=3,0	MS=505,506
71,71,83	G=4,12,12,12,1,1	M=4	LP=3,0	MS=505,506
72,72,84	G=4,12,12,12,1,1	M=4	LP=3,0	MS=505,506
C Interior columns for levels 6-10				
66,66,78	G=4,12,12,12,1,1	M=2	LP=3,0	MS=505,506
67,67,79	G=4,12,12,12,1,1	M=2	LP=3,0	MS=505,506
C Exterior columns for levels 11-13				
121,121,133	G=2,12,12,12,1,1	M=5	LP=3,0	MS=510,511
122,122,134	G=2,12,12,12,1,1	M=5	LP=3,0	MS=510,511
123,123,135	G=2,12,12,12,1,1	M=5	LP=3,0	MS=510,511
124,124,136	G=2,12,12,12,1,1	M=5	LP=3,0	MS=510,511
125,125,137	G=2,12,12,12,1,1	M=5	LP=3,0	MS=510,511
128,128,140	G=2,12,12,12,1,1	M=5	LP=3,0	MS=510,511
129,129,141	G=2,12,12,12,1,1	M=5	LP=3,0	MS=510,511
130,130,142	G=2,12,12,12,1,1	M=5	LP=3,0	MS=510,511
131,131,143	G=2,12,12,12,1,1	M=5	LP=3,0	MS=510,511
132,132,144	G=2,12,12,12,1,1	M=5	LP=3,0	MS=510,511
C Interior columns levels 11-13				
126,126,138	G=2,12,12,12,1,1	M=3	LP=3,0	MS=510,511
127,127,139	G=2,12,12,12,1,1	M=3	LP=3,0	MS=510,511
C Exterior columns levels 14-15				
157,157,169	G=1,12,12,12,1,1	M=6	LP=3,0	MS=513,514
158,158,170	G=1,12,12,12,1,1	M=6	LP=3,0	MS=513,514
159,159,171	G=1,12,12,12,1,1	M=6	LP=3,0	MS=513,514
160,160,172	G=1,12,12,12,1,1	M=6	LP=3,0	MS=513,514
161,161,173	G=1,12,12,12,1,1	M=6	LP=3,0	MS=513,514
164,164,176	G=1,12,12,12,1,1	M=6	LP=3,0	MS=513,514
165,165,177	G=1,12,12,12,1,1	M=6	LP=3,0	MS=513,514
166,166,178	G=1,12,12,12,1,1	M=6	LP=3,0	MS=513,514
167,167,179	G=1,12,12,12,1,1	M=6	LP=3,0	MS=513,514
168,168,180	G=1,12,12,12,1,1	M=6	LP=3,0	MS=513,514
C Interior columns levels 14-15				
162,162,174	G=1,12,12,12,1,1	M=5	LP=3,0	MS=513,514
163,163,175	G=1,12,12,12,1,1	M=5	LP=3,0	MS=513,514
C BEAMS				
C Exterior beams				
181,13,14	G=14,1,12,12,1,1	M=7	LP=2,0	NSL=2 MS=501,501
196,14,15	G=14,1,12,12,1,1	M=7	LP=2,0	NSL=2 MS=501,501
211,15,16	G=14,1,12,12,1,1	M=7	LP=2,0	NSL=2 MS=501,501
271,21,22	G=14,1,12,12,1,1	M=7	LP=2,0	NSL=2 MS=501,501
286,22,23	G=14,1,12,12,1,1	M=7	LP=2,0	NSL=2 MS=501,501
301,23,24	G=14,1,12,12,1,1	M=7	LP=2,0	NSL=2 MS=501,501
316,13,17	G=14,1,12,12,1,1	M=7	LP=3,0	NSL=2 MS=501,501
331,17,21	G=14,1,12,12,1,1	M=7	LP=3,0	NSL=2 MS=501,501
406,16,20	G=14,1,12,12,1,1	M=7	LP=3,0	NSL=2 MS=501,501
421,20,24	G=14,1,12,12,1,1	M=7	LP=3,0	NSL=2 MS=501,501
C Interior beams				
346,14,18	G=14,1,12,12,1,1	M=7	LP=3,0	NSL=1 MS=501,501
361,18,22	G=14,1,12,12,1,1	M=7	LP=3,0	NSL=1 MS=501,501
376,15,19	G=14,1,12,12,1,1	M=7	LP=3,0	NSL=1 MS=501,501
391,19,23	G=14,1,12,12,1,1	M=7	LP=3,0	NSL=1 MS=501,501
226,17,18	G=14,1,12,12,1,1	M=7	LP=2,0	NSL=1 MS=501,501
241,18,19	G=14,1,12,12,1,1	M=7	LP=2,0	NSL=1 MS=501,501
256,19,20	G=14,1,12,12,1,1	M=7	LP=2,0	NSL=1 MS=501,501

LOADS

C Lateral Joint Loads(Earthquake + Wind) in the longitudinal direction

C ie parallel to the y-axis

501 F=0,1.5,0 L=2

502 F=0,3.01,0
503 F=0,4.51,0
504 F=0,6.01,0
505 F=0,7.52,0
506 F=0,8.49,0
507 F=0,9.91,0
508 F=0,11.32,0
509 F=0,12.74,0
510 F=0,14.15,0
511 F=0,14.62,0
512 F=0,15.94,0
513 F=0,17.27,0
514 F=0,17.83,0
515 F=0,19.10,0

C 30% of Lateral Join Loads in the transverse direction
C ie parallel to the x-axis

501 F=0.86,0,0
502 F=1.72,0,0
503 F=2.58,0,0
504 F=3.44,0,0
505 F=4.30,0,0
506 F=4.85,0,0
507 F=5.66,0,0
508 F=6.47,0,0
509 F=7.28,0,0
510 F=8.09,0,0
511 F=8.35,0,0
512 F=9.11,0,0
513 F=9.87,0,0
514 F=10.19,0,0
515 F=10.91,0,0

COMBO

1 C=9.81,9.81

SELECT

NT=1 ID=1,192 SW=1

NT=2 ID=1,192 SW=1

NT=5 ID=1,180 SW=1

10 storey building - Span length 5*5 // Fc=25MPa

Unit :KN-m

SYSTEM

L=2

JOINTS

1	X=0	Y=0	Z=0	
4	X=15	Y=0	Z=0	
121	X=0	Y=0	Z=30	
124	X=15	Y=0	Z=30	Q=1,4,121,124,1,12
5	X=0	Y=5	Z=0	
8	X=15	Y=5	Z=0	
125	X=0	Y=5	Z=30	
128	X=15	Y=5	Z=30	Q=5,8,125,128,1,12
9	X=0	Y=10	Z=0	
12	X=15	Y=10	Z=0	
129	X=0	Y=10	Z=30	
132	X=15	Y=10	Z=30	Q=9,12,129,132,1,12
501	X=7.5	Y=5	Z=3	
510	X=7.5	Y=5	Z=30	G=501,510,1

RESTRAINTS

13,132,1	R=1,1,0,0,0,1	
1,12,1	R=1,1,1,1,1,1	:FIX BASE
501,510,1	R=0,0,1,1,1,0	

FRAME

NM=4 NL=2

C total of 5 member section properties & 2 span loading configurations

1	SH=R	T=0.7,0.7	E=2.4038e7
2	SH=R	T=0.5,0.5	
3	SH=R	T=0.4,0.4	
4	SH=R	T=0.5,0.25	

C Below is the span loading(dead +live)data. Values in t/m length of beam

1 WG=0,0,-4.421

2 WG=0,0,-2.211

C Exterior columns for levels 1-5

1,1,13	G=4,12,12,12,1,1	M=2	LP=3,0	MS=0,501
2,2,14	G=4,12,12,12,1,1	M=2	LP=3,0	MS=0,501
3,3,15	G=4,12,12,12,1,1	M=2	LP=3,0	MS=0,501
4,4,16	G=4,12,12,12,1,1	M=2	LP=3,0	MS=0,501
5,5,17	G=4,12,12,12,1,1	M=2	LP=3,0	MS=0,501
8,8,20	G=4,12,12,12,1,1	M=2	LP=3,0	MS=0,501
9,9,21	G=4,12,12,12,1,1	M=2	LP=3,0	MS=0,501
10,10,22	G=4,12,12,12,1,1	M=2	LP=3,0	MS=0,501
11,11,23	G=4,12,12,12,1,1	M=2	LP=3,0	MS=0,501
12,12,24	G=4,12,12,12,1,1	M=2	LP=3,0	MS=0,501

C Interior columns -levels 1-5

6,6,18	G=4,12,12,12,1,1	M=1	LP=3,0	MS=0,501
7,7,19	G=4,12,12,12,1,1	M=1	LP=3,0	MS=0,501

C Exterior columns for levels 6-10

61,61,73	G=4,12,12,12,1,1	M=3	LP=3,0	MS=505,506
62,62,74	G=4,12,12,12,1,1	M=3	LP=3,0	MS=505,506
63,63,75	G=4,12,12,12,1,1	M=3	LP=3,0	MS=505,506
64,64,76	G=4,12,12,12,1,1	M=3	LP=3,0	MS=505,506
65,65,77	G=4,12,12,12,1,1	M=3	LP=3,0	MS=505,506
68,68,80	G=4,12,12,12,1,1	M=3	LP=3,0	MS=505,506
69,69,81	G=4,12,12,12,1,1	M=3	LP=3,0	MS=505,506
70,70,82	G=4,12,12,12,1,1	M=3	LP=3,0	MS=505,506
71,71,83	G=4,12,12,12,1,1	M=3	LP=3,0	MS=505,506
72,72,84	G=4,12,12,12,1,1	M=3	LP=3,0	MS=505,506

C Interior columns for levels 6-10

66,66,78	G=4,12,12,12,1,1	M=2	LP=3,0	MS=505,506
67,67,79	G=4,12,12,12,1,1	M=2	LP=3,0	MS=505,506

C BEAMS

C Exterior beams

121,13,14	G=9,1,12,12,1,1	M=4	LP=2,0	NSL=2	MS=501,501
131,14,15	G=9,1,12,12,1,1	M=4	LP=2,0	NSL=2	MS=501,501
141,15,16	G=9,1,12,12,1,1	M=4	LP=2,0	NSL=2	MS=501,501
181,21,22	G=9,1,12,12,1,1	M=4	LP=2,0	NSL=2	MS=501,501
191,22,23	G=9,1,12,12,1,1	M=4	LP=2,0	NSL=2	MS=501,501
201,23,24	G=9,1,12,12,1,1	M=4	LP=2,0	NSL=2	MS=501,501
211,13,17	G=9,1,12,12,1,1	M=4	LP=3,0	NSL=2	MS=501,501
221,17,21	G=9,1,12,12,1,1	M=4	LP=3,0	NSL=2	MS=501,501
271,16,20	G=9,1,12,12,1,1	M=4	LP=3,0	NSL=2	MS=501,501
281,20,24	G=9,1,12,12,1,1	M=4	LP=3,0	NSL=2	MS=501,501

C Interior beams

231,14,18	G=9,1,12,12,1,1	M=4	LP=3,0	NSL=1	MS=501,501
241,18,22	G=9,1,12,12,1,1	M=4	LP=3,0	NSL=1	MS=501,501
251,15,19	G=9,1,12,12,1,1	M=4	LP=3,0	NSL=1	MS=501,501
261,19,23	G=9,1,12,12,1,1	M=4	LP=3,0	NSL=1	MS=501,501
151,17,18	G=9,1,12,12,1,1	M=4	LP=2,0	NSL=1	MS=501,501
161,18,19	G=9,1,12,12,1,1	M=4	LP=2,0	NSL=1	MS=501,501
171,19,20	G=9,1,12,12,1,1	M=4	LP=2,0	NSL=1	MS=501,501

LOADS

C Lateral Joint Loads (Earthquake + Wind) in the longitudinal direction
C ie parallel to the y-axis

501	F=0,2.58,0	L=2
502	F=0,5.15,0	
503	F=0,7.73,0	
504	F=0,10.30,0	
505	F=0,12.88,0	
506	F=0,14.51,0	
507	F=0,16.93,0	
508	F=0,19.34,0	
509	F=0,21.76,0	
510	F=0,24.18,0	

C 30% of Lateral Join Loads in the transverse direction

C ie parallel to the x-axis

501	F=1.45,0,0
502	F=2.89,0,0
503	F=4.34,0,0
504	F=5.79,0,0
505	F=7.24,0,0
506	F=8.15,0,0
507	F=9.51,0,0
508	F=10.87,0,0
509	F=12.23,0,0
510	F=13.58,0,0

COMBO

1 C=9.81,9.81

SELECT

NT=1 ID=1,132 SW=1
NT=2 ID=1,132 SW=1
NT=5 ID=1,120 SW=1

5 storey building-Span length 5*5 //Fc=25MPa

Unit :KN-m

SYSTEM

L=2

JOINTS

1	X=0	Y=0	Z=0	
4	X=15	Y=0	Z=0	
61	X=0	Y=0	Z=15	
64	X=15	Y=0	Z=15	Q=1,4,61,64,1,12
5	X=0	Y=5	Z=0	
8	X=15	Y=5	Z=0	
65	X=0	Y=5	Z=15	
68	X=15	Y=5	Z=15	Q=5,8,65,68,1,12
9	X=0	Y=10	Z=0	
12	X=15	Y=10	Z=0	
69	X=0	Y=10	Z=15	
72	X=15	Y=10	Z=15	Q=9,12,69,72,1,12
501	X=7.5	Y=5	Z=3	
505	X=7.5	Y=5	Z=15	G=501,505,1

RESTRAINTS

13,72,1	R=1,1,0,0,0,1	
1,12,1	R=1,1,1,1,1,1	:FIX BASE
501,505,1	R=0,0,1,1,1,0	

FRAME

NM=3 NL=2

C total of 8 member section properties & 2 span loading configurations

1	SH=R	T=0.5,0.5	E=2.4038e7
2	SH=R	T=0.4,0.4	
3	SH=R	T=0.5,0.25	

C Below is the span loading(dead +live)data. Values in t/m length of beam

1	WG=0,0,-4.421
2	WG=0,0,-2.211

C Exterior columns for levels 1-2

1,1,13	G=1,12,12,12,1,1	M=2	LP=3,0	MS=0,501
2,2,14	G=1,12,12,12,1,1	M=2	LP=3,0	MS=0,501
3,3,15	G=1,12,12,12,1,1	M=2	LP=3,0	MS=0,501
4,4,16	G=1,12,12,12,1,1	M=2	LP=3,0	MS=0,501
5,5,17	G=1,12,12,12,1,1	M=2	LP=3,0	MS=0,501
8,8,20	G=1,12,12,12,1,1	M=2	LP=3,0	MS=0,501
9,9,21	G=1,12,12,12,1,1	M=2	LP=3,0	MS=0,501
10,10,22	G=1,12,12,12,1,1	M=2	LP=3,0	MS=0,501
11,11,23	G=1,12,12,12,1,1	M=2	LP=3,0	MS=0,501
12,12,24	G=1,12,12,12,1,1	M=2	LP=3,0	MS=0,501

C Interior columns -levels 1-2

6,6,18	G=1,12,12,12,1,1	M=1	LP=3,0	MS=0,501
7,7,19	G=1,12,12,12,1,1	M=1	LP=3,0	MS=0,501

C Exterior columns for levels 3-5

25,25,37	G=2,12,12,12,1,1	M=2	LP=3,0	MS=502,503
26,26,38	G=2,12,12,12,1,1	M=2	LP=3,0	MS=502,503
27,27,39	G=2,12,12,12,1,1	M=2	LP=3,0	MS=502,503
28,28,40	G=2,12,12,12,1,1	M=2	LP=3,0	MS=502,503
29,29,41	G=2,12,12,12,1,1	M=2	LP=3,0	MS=502,503
32,32,44	G=2,12,12,12,1,1	M=2	LP=3,0	MS=502,503
33,33,45	G=2,12,12,12,1,1	M=2	LP=3,0	MS=502,503
34,34,46	G=2,12,12,12,1,1	M=2	LP=3,0	MS=502,503
35,35,47	G=2,12,12,12,1,1	M=2	LP=3,0	MS=502,503
36,36,48	G=2,12,12,12,1,1	M=2	LP=3,0	MS=502,503

C Interior columns for levels 3-5

30,30,42	G=2,12,12,12,1,1	M=1	LP=3,0	MS=502,503
31,31,43	G=2,12,12,12,1,1	M=1	LP=3,0	MS=502,503

C BEAMS

C Exterior beams

61,13,14	G=4,1,12,12,1,1	M=3	LP=2,0	NSL=2	MS=501,501
66,14,15	G=4,1,12,12,1,1	M=3	LP=2,0	NSL=2	MS=501,501
71,15,16	G=4,1,12,12,1,1	M=3	LP=2,0	NSL=2	MS=501,501
91,21,22	G=4,1,12,12,1,1	M=3	LP=2,0	NSL=2	MS=501,501
96,22,23	G=4,1,12,12,1,1	M=3	LP=2,0	NSL=2	MS=501,501
101,23,24	G=4,1,12,12,1,1	M=3	LP=2,0	NSL=2	MS=501,501
106,13,17	G=4,1,12,12,1,1	M=3	LP=3,0	NSL=2	MS=501,501
111,17,21	G=4,1,12,12,1,1	M=3	LP=3,0	NSL=2	MS=501,501
136,16,20	G=4,1,12,12,1,1	M=3	LP=3,0	NSL=2	MS=501,501
141,20,24	G=4,1,12,12,1,1	M=3	LP=3,0	NSL=2	MS=501,501

C Interior beams

116,14,18	G=4,1,12,12,1,1	M=3	LP=3,0	NSL=1	MS=501,501
121,18,22	G=4,1,12,12,1,1	M=3	LP=3,0	NSL=1	MS=501,501
126,15,19	G=4,1,12,12,1,1	M=3	LP=3,0	NSL=1	MS=501,501
131,19,23	G=4,1,12,12,1,1	M=3	LP=3,0	NSL=1	MS=501,501
76,17,18	G=4,1,12,12,1,1	M=3	LP=2,0	NSL=1	MS=501,501
81,18,19	G=4,1,12,12,1,1	M=3	LP=2,0	NSL=1	MS=501,501
86,19,20	G=4,1,12,12,1,1	M=3	LP=2,0	NSL=1	MS=501,501

LOADS

C Lateral Joint Loads (Earthquake + Wind) in the longitudinal direction

C ie parallel to the y-axis

501	F=0,4.91,0	L=2
502	F=0,9.82,0	
503	F=0,14.73,0	
504	F=0,19.64,0	
505	F=0,24.55,0	

C 30% of Lateral Join Loads in the transverse direction

C ie parallel to the x-axis

501	F=3.49,0,0
502	F=6.97,0,0
503	F=10.46,0,0
504	F=13.94,0,0
505	F=17.43,0,0

COMBO

1 C=9.81,9.81

SELECT

NT=1	ID=1,72	SW=1
NT=2	ID=1,72	SW=1
NT=5	ID=1,60	SW=1

PROGRAM:SAP90/FILE:\1a.F3F

20 storey building - Bay size 5*5// Fc=25MPa

FRAME ELEMENT FORCES

ELT ID	LOAD COMB	DIST ENDI	1-2 PLANE		AXIAL FORCE	1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT		SHEAR	MOMENT	

1	1	.000			-666.862			.000
		.000	-9.166	-43.505		9.776	13.510	
		3.000	-9.166	-71.003		9.776	42.838	
		3.000			-666.862			.000

2	1	.000			-2714.692			.000
		.000	-17.733	-35.666		-.852	23.236	
		3.000	-17.733	-88.864		-.852	20.679	
		3.000			-2714.692			.000

3	1	.000			-2826.277			.000
		.000	-17.750	-35.650		-2.256	24.520	
		3.000	-17.750	-88.900		-2.256	17.753	
		3.000			-2826.277			.000

4	1	.000			-1959.248			.000
		.000	-9.204	-43.470		-9.971	31.580	
		3.000	-9.204	-71.083		-9.971	1.667	
		3.000			-1959.248			.000

5	1	.000			-3325.917			.000
		.000	3.414	-55.017		18.357	5.658	
		3.000	3.414	-44.774		18.357	60.728	
		3.000			-3325.917			.000

6	1	.000			-8280.147			.000
		.000	-.469	-141.399		.704	60.758	
		3.000	-.469	-142.807		.704	62.871	
		3.000			-8280.147			.000

7	1	.000			-8511.326			.000
		.000	-.469	-141.399		.043	61.326	
		3.000	-.469	-142.807		.043	61.456	
		3.000			-8511.326			.000

8	1	.000			-4681.717			.000
		.000	3.414	-55.017		-18.579	39.457	
		3.000	3.414	-44.774		-18.579	-16.281	
		3.000			-4681.717			.000

9	1	.000			-3347.776			.000
		.000	10.432	-61.439		9.862	13.432	
		3.000	10.432	-30.142		9.862	43.017	
		3.000			-3347.776			.000

10	1	.000			-5540.653			.000
		.000	19.015	-69.293		-.768	23.159	
		3.000	19.015	-12.247		-.768	20.855	
		3.000			-5540.653			.000

11	1	.000			-5652.238			.000
		.000	19.032	-69.308		-2.340	24.598	
		3.000	19.032	-12.211		-2.340	17.576	
		3.000			-5652.238			.000

12	1	.000			-4640.163			.000
		.000	10.471	-61.474		-10.057	31.659	
		3.000	10.471	-30.062		-10.057	1.488	
		3.000			-4640.163			.000

13	1	.000			-565.228			.000

		.000	-17.246	-40.559	18.754	1.619	
		3.000	-17.246	-92.296	18.754	57.882	
		3.000		-565.228			.000
14		-----					
	1	.000		-2506.541			.000
		.000	-32.190	-16.862	-2.400	33.410	
		3.000	-32.190	-113.433	-2.400	26.210	
		3.000		-2506.541			.000
15		-----					
	1	.000		-2616.817			.000
		.000	-32.241	-16.795	-6.918	39.186	
		3.000	-32.241	-113.519	-6.918	18.432	
		3.000		-2616.817			.000
16		-----					
	1	.000		-1851.288			.000
		.000	-17.359	-40.410	-18.277	58.606	
		3.000	-17.359	-92.489	-18.277	3.776	
		3.000		-1851.288			.000
17		-----					
	1	.000		-3113.999			.000
		.000	10.440	-80.611	33.769	-22.159	
		3.000	10.440	-49.290	33.769	79.149	
		3.000		-3113.999			.000
18		-----					
	1	.000		-7848.426			.000
		.000	-3.699	-179.477	3.986	78.243	
		3.000	-3.699	-190.573	3.986	90.200	
		3.000		-7848.426			.000
19		-----					
	1	.000		-8078.270			.000
		.000	-3.699	-179.477	1.131	81.543	
		3.000	-3.699	-190.573	1.131	84.936	
		3.000		-8078.270			.000
20		-----					
	1	.000		-4463.621			.000
		.000	10.440	-80.611	-33.369	82.487	
		3.000	10.440	-49.290	-33.369	-17.620	
		3.000		-4463.621			.000
21		-----					
	1	.000		-3231.483			.000
		.000	19.350	-96.974	19.011	1.283	
		3.000	19.350	-38.924	19.011	58.316	
		3.000		-3231.483			.000
22		-----					
	1	.000		-5318.087			.000
		.000	34.332	-120.724	-2.147	33.080	
		3.000	34.332	-17.729	-2.147	26.638	
		3.000		-5318.087			.000
23		-----					
	1	.000		-5428.364			.000
		.000	34.382	-120.791	-7.171	39.517	
		3.000	34.382	-17.643	-7.171	18.004	
		3.000		-5428.364			.000
24		-----					
	1	.000		-4517.542			.000
		.000	19.464	-97.123	-18.533	58.941	
		3.000	19.464	-38.731	-18.533	3.342	
		3.000		-4517.542			.000
25		-----					
	1	.000		-474.282			.000
		.000	-16.634	-82.383	19.358	23.011	
		3.000	-16.634	-132.284	19.358	81.086	
		3.000		-474.282			.000
26		-----					
	1	.000		-2309.768			.000
		.000	-29.586	-63.070	-5.682	58.240	
		3.000	-29.586	-151.828	-5.682	41.192	

		3.000		-2309.768			.000
27	-----						
	1	.000		-2417.488			.000
		.000	-29.673	-62.951	-13.402	68.947	
		3.000	-29.673	-151.969	-13.402	28.742	
		3.000		-2417.488			.000
28	-----						
	1	.000		-1744.803			.000
		.000	-16.826	-82.119	-17.924	77.347	
		3.000	-16.826	-132.597	-17.924	23.576	
		3.000		-1744.803			.000
29	-----						
	1	.000		-2908.792			.000
		.000	21.409	-134.867	32.521	3.440	
		3.000	21.409	-70.640	32.521	101.002	
		3.000		-2908.792			.000
30	-----						
	1	.000		-7421.953			.000
		.000	-11.825	-278.243	10.638	126.642	
		3.000	-11.825	-313.717	10.638	158.557	
		3.000		-7421.953			.000
31	-----						
	1	.000		-7649.128			.000
		.000	-11.825	-278.243	5.141	134.177	
		3.000	-11.825	-313.717	5.141	149.601	
		3.000		-7649.128			.000
32	-----						
	1	.000		-4243.112			.000
		.000	21.409	-134.867	-31.345	97.214	
		3.000	21.409	-70.640	-31.345	3.179	
		3.000		-4243.112			.000
33	-----						
	1	.000		-3105.611			.000
		.000	19.925	-135.722	19.792	22.416	
		3.000	19.925	-75.946	19.792	81.790	
		3.000		-3105.611			.000
34	-----						
	1	.000		-5086.763			.000
		.000	33.191	-155.346	-5.256	57.652	
		3.000	33.191	-55.773	-5.256	41.885	
		3.000		-5086.763			.000
35	-----						
	1	.000		-5194.483			.000
		.000	33.278	-155.465	-13.829	69.534	
		3.000	33.278	-55.631	-13.829	28.049	
		3.000		-5194.483			.000
36	-----						
	1	.000		-4376.132			.000
		.000	20.118	-135.986	-18.357	77.943	
		3.000	20.118	-75.634	-18.357	22.872	
		3.000		-4376.132			.000
37	-----						
	1	.000		-405.337			.000
		.000	-18.975	-161.422	22.541	62.703	
		3.000	-18.975	-218.347	22.541	130.327	
		3.000		-405.337			.000
38	-----						
	1	.000		-2131.640			.000
		.000	-31.435	-142.329	-12.073	111.358	
		3.000	-31.435	-236.635	-12.073	75.138	
		3.000		-2131.640			.000
39	-----						
	1	.000		-2235.714			.000
		.000	-31.553	-142.159	-22.240	126.049	
		3.000	-31.553	-236.819	-22.240	59.328	
		3.000		-2235.714			.000
40	-----						

	1	.000		-1643.680			.000
		.000	-19.234	-161.049	-17.060	122.336	
		3.000	-19.234	-218.751	-17.060	71.157	
		3.000		-1643.680			.000
41	-----						
	1	.000		-2713.912			.000
		.000	38.510	-240.858	35.303	43.155	
		3.000	38.510	-125.328	35.303	149.063	
		3.000		-2713.912			.000
42	-----						
	1	.000		-6999.774			.000
		.000	-18.382	-489.088	17.864	236.112	
		3.000	-18.382	-544.233	17.864	289.706	
		3.000		-6999.774			.000
43	-----						
	1	.000		-7222.845			.000
		.000	-18.382	-489.088	10.290	246.853	
		3.000	-18.382	-544.233	10.290	277.725	
		3.000		-7222.845			.000
44	-----						
	1	.000		-4016.124			.000
		.000	38.510	-240.858	-30.220	142.494	
		3.000	38.510	-125.328	-30.220	51.834	
		3.000		-4016.124			.000
45	-----						
	1	.000		-2967.393			.000
		.000	19.647	-219.648	23.125	61.862	
		3.000	19.647	-160.708	23.125	131.237	
		3.000		-2967.393			.000
46	-----						
	1	.000		-4839.368			.000
		.000	32.610	-239.544	-11.498	110.529	
		3.000	32.610	-141.715	-11.498	76.036	
		3.000		-4839.368			.000
47	-----						
	1	.000		-4943.442			.000
		.000	32.728	-239.714	-22.816	126.878	
		3.000	32.728	-141.531	-22.816	58.430	
		3.000		-4943.442			.000
48	-----						
	1	.000		-4205.736			.000
		.000	19.906	-220.022	-17.644	123.178	
		3.000	19.906	-160.304	-17.644	70.247	
		3.000		-4205.736			.000
49	-----						
	1	.000		-381.543			.000
		.000	97.441	-324.194	-37.462	148.914	
		3.000	97.441	-31.871	-37.462	36.529	
		3.000		-381.543			.000
50	-----						
	1	.000		-1986.852			.000
		.000	84.384	-305.360	-93.376	223.594	
		3.000	84.384	-52.209	-93.376	-56.533	
		3.000		-1986.852			.000
51	-----						
	1	.000		-2086.525			.000
		.000	84.205	-305.120	-107.797	243.297	
		3.000	84.205	-52.506	-107.797	-80.093	
		3.000		-2086.525			.000
52	-----						
	1	.000		-1554.838			.000
		.000	97.060	-323.679	-85.122	215.460	
		3.000	97.060	-32.499	-85.122	-39.906	
		3.000		-1554.838			.000
53	-----						
	1	.000		-2537.330			.000
		.000	195.532	-454.371	-23.675	129.241	

		3.000	195.532	132.224	-23.675	58.215	
		3.000		-2537.330			.000
54		-----					
	1	.000		-6581.968			.000
		.000	254.503	-881.793	-127.912	448.624	
		3.000	254.503	-118.284	-127.912	64.888	
		3.000		-6581.968			.000
55		-----					
	1	.000		-6799.791			.000
		.000	254.503	-881.793	-138.691	463.360	
		3.000	254.503	-118.284	-138.691	47.286	
		3.000		-6799.791			.000
56		-----					
	1	.000		-3774.322			.000
		.000	195.532	-454.371	-100.107	236.452	
		3.000	195.532	132.224	-100.107	-63.870	
		3.000		-3774.322			.000
57		-----					
	1	.000		-2810.103			.000
		.000	143.694	-388.816	-36.605	147.754	
		3.000	143.694	42.265	-36.605	37.941	
		3.000		-2810.103			.000
58		-----					
	1	.000		-4560.635			.000
		.000	158.653	-409.637	-92.531	222.450	
		3.000	158.653	66.324	-92.531	-55.143	
		3.000		-4560.635			.000
59		-----					
	1	.000		-4660.309			.000
		.000	158.832	-409.876	-108.641	244.440	
		3.000	158.832	66.621	-108.641	-81.483	
		3.000		-4660.309			.000
60		-----					
	1	.000		-3983.398			.000
		.000	144.075	-389.331	-85.979	216.620	
		3.000	144.075	42.892	-85.979	-41.318	
		3.000		-3983.398			.000
61		-----					
	1	.000		-398.999			.000
		.000	93.290	-208.069	-37.001	89.899	
		3.000	93.290	71.802	-37.001	-21.105	
		3.000		-398.999			.000
62		-----					
	1	.000		-1873.204			.000
		.000	84.619	-195.410	-90.179	162.373	
		3.000	84.619	58.446	-90.179	-108.163	
		3.000		-1873.204			.000
63		-----					
	1	.000		-1968.531			.000
		.000	84.443	-195.173	-103.651	180.599	
		3.000	84.443	58.156	-103.651	-130.354	
		3.000		-1968.531			.000
64		-----					
	1	.000		-1477.342			.000
		.000	92.908	-207.559	-77.261	145.364	
		3.000	92.908	71.164	-77.261	-86.419	
		3.000		-1477.342			.000
65		-----					
	1	.000		-2378.022			.000
		.000	189.159	-338.080	-27.073	75.695	
		3.000	189.159	229.397	-27.073	-5.525	
		3.000		-2378.022			.000
66		-----					
	1	.000		-6167.537			.000
		.000	262.642	-610.722	-132.199	303.659	
		3.000	262.642	177.203	-132.199	-92.938	
		3.000		-6167.537			.000

67	1	.000			-6378.283		.000
		.000	262.642	-610.722	-142.594	318.105	
		3.000	262.642	177.203	-142.594	-109.676	
		3.000			-6378.283		.000
68	1	.000			-3518.226		.000
		.000	189.159	-338.080	-89.339	162.222	
		3.000	189.159	229.397	-89.339	-105.797	
		3.000			-3518.226		.000
69	1	.000			-2636.274		.000
		.000	132.261	-261.779	-36.149	88.761	
		3.000	132.261	135.004	-36.149	-19.685	
		3.000			-2636.274		.000
70	1	.000			-4251.951		.000
		.000	144.768	-279.103	-89.347	161.260	
		3.000	144.768	155.202	-89.347	-106.780	
		3.000			-4251.951		.000
71	1	.000			-4347.277		.000
		.000	144.944	-279.340	-104.483	181.712	
		3.000	144.944	155.492	-104.483	-131.737	
		3.000			-4347.277		.000
72	1	.000			-3714.617		.000
		.000	132.643	-262.288	-78.113	146.501	
		3.000	132.643	135.642	-78.113	-87.839	
		3.000			-3714.617		.000
73	1	.000			-440.454		.000
		.000	84.844	-147.931	-28.956	51.734	
		3.000	84.844	106.602	-28.956	-35.136	
		3.000			-440.454		.000
74	1	.000			-1780.632		.000
		.000	75.965	-134.381	-93.745	149.678	
		3.000	75.965	93.514	-93.745	-131.557	
		3.000			-1780.632		.000
75	1	.000			-1871.928		.000
		.000	75.731	-134.036	-111.711	176.083	
		3.000	75.731	93.157	-111.711	-159.050	
		3.000			-1871.928		.000
76	1	.000			-1406.169		.000
		.000	84.317	-147.158	-77.685	124.988	
		3.000	84.317	105.794	-77.685	-108.067	
		3.000			-1406.169		.000
77	1	.000			-2230.707		.000
		.000	203.466	-326.860	-18.376	35.542	
		3.000	203.466	283.540	-18.376	-19.585	
		3.000			-2230.707		.000
78	1	.000			-5755.260		.000
		.000	233.997	-417.371	-113.216	196.702	
		3.000	233.997	284.620	-113.216	-142.946	
		3.000			-5755.260		.000
79	1	.000			-5956.920		.000
		.000	233.997	-417.371	-126.265	215.800	
		3.000	233.997	284.620	-126.265	-162.995	
		3.000			-5956.920		.000
80	1	.000			-3254.593		.000

		.000	203.466	-326.860	-91.101	145.620		
		3.000	203.466	283.540	-91.101	-127.682		
		3.000		-3254.593			.000	
81		-----						
	1	.000		-2450.283			.000	
		.000	131.832	-218.650	-27.782	50.012		
		3.000	131.832	176.847	-27.782	-33.334		
		3.000		-2450.283			.000	
82		-----						
	1	.000		-3924.335			.000	
		.000	145.781	-240.200	-92.602	148.004		
		3.000	145.781	197.143	-92.602	-129.801		
		3.000		-3924.335			.000	
83		-----						
	1	.000		-4015.631			.000	
		.000	146.015	-240.545	-112.854	177.757		
		3.000	146.015	197.500	-112.854	-160.806		
		3.000		-4015.631			.000	
84		-----						
	1	.000		-3415.999			.000	
		.000	132.360	-219.424	-78.859	126.710		
		3.000	132.360	177.655	-78.859	-109.868		
		3.000		-3415.999			.000	
85		-----						
	1	.000		-485.632			.000	
		.000	79.957	-121.769	-25.628	38.143		
		3.000	79.957	118.102	-25.628	-38.741		
		3.000		-485.632			.000	
86		-----						
	1	.000		-1694.653			.000	
		.000	71.743	-109.305	-89.782	133.969		
		3.000	71.743	105.923	-89.782	-135.377		
		3.000		-1694.653			.000	
87		-----						
	1	.000		-1780.340			.000	
		.000	71.476	-108.917	-110.196	163.758		
		3.000	71.476	105.511	-110.196	-166.831		
		3.000		-1780.340			.000	
88		-----						
	1	.000		-1334.592			.000	
		.000	79.345	-120.881	-76.209	112.811		
		3.000	79.345	117.153	-76.209	-115.816		
		3.000		-1334.592			.000	
89		-----						
	1	.000		-2087.346			.000	
		.000	198.437	-298.573	-15.731	23.233		
		3.000	198.437	296.737	-15.731	-23.959		
		3.000		-2087.346			.000	
90		-----						
	1	.000		-5346.589			.000	
		.000	219.835	-334.448	-105.130	155.124		
		3.000	219.835	325.056	-105.130	-160.265		
		3.000		-5346.589			.000	
91		-----						
	1	.000		-5537.886			.000	
		.000	219.835	-334.448	-119.851	176.659		
		3.000	219.835	325.056	-119.851	-182.894		
		3.000		-5537.886			.000	
92		-----						
	1	.000		-2991.293			.000	
		.000	198.437	-298.573	-88.786	131.640		
		3.000	198.437	296.737	-88.786	-134.717		
		3.000		-2991.293			.000	
93		-----						
	1	.000		-2257.529			.000	
		.000	128.507	-193.479	-24.262	36.162		
		3.000	128.507	192.043	-24.262	-36.625		

94		3.000		-2257.529			.000
	1	.000		-3593.763			.000
		.000	141.405	-212.790	-88.452	132.038	
		3.000	141.405	211.426	-88.452	-133.317	
		3.000		-3593.763			.000
95							
	1	.000		-3679.450			.000
		.000	141.672	-213.177	-111.526	165.688	
		3.000	141.672	211.838	-111.526	-168.891	
		3.000		-3679.450			.000
96							
	1	.000		-3106.488			.000
		.000	129.119	-194.366	-77.574	114.791	
		3.000	129.119	192.992	-77.574	-117.932	
		3.000		-3106.488			.000
97							
	1	.000		-525.426			.000
		.000	74.384	-103.475	-22.413	28.069	
		3.000	74.384	119.676	-22.413	-39.169	
		3.000		-525.426			.000
98							
	1	.000		-1608.618			.000
		.000	66.520	-91.625	-84.013	122.311	
		3.000	66.520	107.935	-84.013	-129.728	
		3.000		-1608.618			.000
99							
	1	.000		-1687.531			.000
		.000	66.229	-91.190	-106.199	155.426	
		3.000	66.229	107.497	-106.199	-163.170	
		3.000		-1687.531			.000
100							
	1	.000		-1259.747			.000
		.000	73.710	-102.468	-73.949	106.040	
		3.000	73.710	118.660	-73.949	-115.808	
		3.000		-1259.747			.000
101							
	1	.000		-1944.633			.000
		.000	188.637	-278.148	-13.034	13.774	
		3.000	188.637	287.764	-13.034	-25.329	
		3.000		-1944.633			.000
102							
	1	.000		-4940.753			.000
		.000	210.199	-286.777	-99.782	131.310	
		3.000	210.199	343.821	-99.782	-168.036	
		3.000		-4940.753			.000
103							
	1	.000		-5120.579			.000
		.000	210.199	-286.777	-115.834	155.121	
		3.000	210.199	343.821	-115.834	-192.380	
		3.000		-5120.579			.000
104							
	1	.000		-2731.083			.000
		.000	188.637	-278.148	-85.615	124.087	
		3.000	188.637	287.764	-85.615	-132.758	
		3.000		-2731.083			.000
105							
	1	.000		-2061.996			.000
		.000	123.646	-178.084	-20.904	25.819	
		3.000	123.646	192.855	-20.904	-36.892	
		3.000		-2061.996			.000
106							
	1	.000		-3266.307			.000
		.000	135.344	-196.341	-82.541	120.118	
		3.000	135.344	209.690	-82.541	-127.505	
		3.000		-3266.307			.000
107							

	1	.000		-3345.220			.000
		.000	135.635	-196.776	-107.671	157.619	
		3.000	135.635	210.128	-107.671	-165.394	
		3.000		-3345.220			.000
108	-----						
	1	.000		-2796.317			.000
		.000	124.320	-179.091	-75.458	108.290	
		3.000	124.320	193.870	-75.458	-118.084	
		3.000		-2796.317			.000
109	-----						
	1	.000		-556.432			.000
		.000	64.875	-88.825	-13.762	18.017	
		3.000	64.875	105.799	-13.762	-23.270	
		3.000		-556.432			.000
110	-----						
	1	.000		-1519.636			.000
		.000	56.490	-76.595	-82.610	115.372	
		3.000	56.490	92.874	-82.610	-132.458	
		3.000		-1519.636			.000
111	-----						
	1	.000		-1590.246			.000
		.000	56.109	-76.073	-110.512	154.173	
		3.000	56.109	92.255	-110.512	-177.362	
		3.000		-1590.246			.000
112	-----						
	1	.000		-1180.366			.000
		.000	64.003	-87.624	-75.645	104.356	
		3.000	64.003	104.386	-75.645	-122.579	
		3.000		-1180.366			.000
113	-----						
	1	.000		-1801.033			.000
		.000	192.878	-269.804	-3.446	3.298	
		3.000	192.878	308.828	-3.446	-7.041	
		3.000		-1801.033			.000
114	-----						
	1	.000		-4538.010			.000
		.000	177.223	-245.175	-79.577	108.855	
		3.000	177.223	286.494	-79.577	-129.875	
		3.000		-4538.010			.000
115	-----						
	1	.000		-4705.747			.000
		.000	177.223	-245.175	-99.064	136.254	
		3.000	177.223	286.494	-99.064	-160.939	
		3.000		-4705.747			.000
116	-----						
	1	.000		-2475.254			.000
		.000	192.878	-269.804	-88.863	122.762	
		3.000	192.878	308.828	-88.863	-143.826	
		3.000		-2475.254			.000
117	-----						
	1	.000		-1864.767			.000
		.000	123.890	-171.147	-11.813	15.327	
		3.000	123.890	200.522	-11.813	-20.112	
		3.000		-1864.767			.000
118	-----						
	1	.000		-2945.029			.000
		.000	137.073	-189.376	-80.712	112.749	
		3.000	137.073	221.842	-80.712	-129.387	
		3.000		-2945.029			.000
119	-----						
	1	.000		-3015.639			.000
		.000	137.453	-189.897	-112.410	156.796	
		3.000	137.453	222.461	-112.410	-180.433	
		3.000		-3015.639			.000
120	-----						
	1	.000		-2488.700			.000
		.000	124.761	-172.348	-77.594	107.046	

		3.000	124.761	201.935	-77.594	-125.737	
		3.000		-2488.700			.000
121		-----					
	1	.000		-576.384			.000
		.000	61.695	-86.212	-17.663	25.176	
		3.000	61.695	98.874	-17.663	-27.814	
		3.000		-576.384			.000
122		-----					
	1	.000		-1426.390			.000
		.000	57.796	-80.040	-66.716	93.629	
		3.000	57.796	93.347	-66.716	-106.518	
		3.000		-1426.390			.000
123		-----					
	1	.000		-1489.239			.000
		.000	57.478	-79.609	-88.396	123.409	
		3.000	57.478	92.825	-88.396	-141.778	
		3.000		-1489.239			.000
124		-----					
	1	.000		-1096.332			.000
		.000	60.957	-85.218	-66.025	91.354	
		3.000	60.957	97.653	-66.025	-106.722	
		3.000		-1096.332			.000
125		-----					
	1	.000		-1656.381			.000
		.000	152.551	-213.164	-11.234	15.863	
		3.000	152.551	244.488	-11.234	-17.838	
		3.000		-1656.381			.000
126		-----					
	1	.000		-4136.183			.000
		.000	202.125	-281.672	-98.923	137.777	
		3.000	202.125	324.703	-98.923	-158.993	
		3.000		-4136.183			.000
127		-----					
	1	.000		-4290.249			.000
		.000	202.125	-281.672	-114.886	160.437	
		3.000	202.125	324.703	-114.886	-184.220	
		3.000		-4290.249			.000
128		-----					
	1	.000		-2223.018			.000
		.000	152.551	-213.164	-76.272	105.426	
		3.000	152.551	244.488	-76.272	-123.389	
		3.000		-2223.018			.000
129		-----					
	1	.000		-1669.438			.000
		.000	108.015	-149.521	-16.039	22.981	
		3.000	108.015	174.525	-16.039	-25.137	
		3.000		-1669.438			.000
130		-----					
	1	.000		-2628.830			.000
		.000	118.840	-164.198	-65.169	91.527	
		3.000	118.840	192.323	-65.169	-103.980	
		3.000		-2628.830			.000
131		-----					
	1	.000		-2691.678			.000
		.000	119.158	-164.628	-89.942	125.511	
		3.000	119.158	192.845	-89.942	-144.316	
		3.000		-2691.678			.000
132		-----					
	1	.000		-2189.386			.000
		.000	108.754	-150.515	-67.649	93.549	
		3.000	108.754	175.746	-67.649	-109.399	
		3.000		-2189.386			.000
133		-----					
	1	.000		-580.333			.000
		.000	53.129	-68.913	-10.442	9.186	
		3.000	53.129	90.473	-10.442	-22.142	
		3.000		-580.333			.000

134	1	.000			-1326.421			.000
		.000	49.402		-63.553	-63.275	91.507	
		3.000	49.402		84.655	-63.275	-98.319	
		3.000			-1326.421			.000
135	1	.000			-1382.827			.000
		.000	49.018		-62.966	-89.668	131.275	
		3.000	49.018		84.086	-89.668	-137.728	
		3.000			-1382.827			.000
136	1	.000			-1006.128			.000
		.000	52.216		-67.526	-66.962	95.898	
		3.000	52.216		89.123	-66.962	-104.988	
		3.000			-1006.128			.000
137	1	.000			-1510.050			.000
		.000	150.995		-221.235	-3.854	-.942	
		3.000	150.995		231.750	-3.854	-12.504	
		3.000			-1510.050			.000
138	1	.000			-3732.606			.000
		.000	168.709		-210.513	-79.873	96.537	
		3.000	168.709		295.613	-79.873	-143.081	
		3.000			-3732.606			.000
139	1	.000			-3870.663			.000
		.000	168.709		-210.513	-97.547	123.331	
		3.000	168.709		295.613	-97.547	-169.310	
		3.000			-3870.663			.000
140	1	.000			-1976.240			.000
		.000	150.995		-221.235	-77.791	113.123	
		3.000	150.995		231.750	-77.791	-120.251	
		3.000			-1976.240			.000
141	1	.000			-1478.638			.000
		.000	107.131		-151.914	-8.432	6.139	
		3.000	107.131		169.479	-8.432	-19.156	
		3.000			-1478.638			.000
142	1	.000			-2320.707			.000
		.000	118.389		-170.126	-61.360	88.616	
		3.000	118.389		185.040	-61.360	-95.463	
		3.000			-2320.707			.000
143	1	.000			-2377.113			.000
		.000	118.774		-170.712	-91.583	134.166	
		3.000	118.774		185.608	-91.583	-140.584	
		3.000			-2377.113			.000
144	1	.000			-1904.433			.000
		.000	108.043		-153.301	-68.973	98.945	
		3.000	108.043		170.828	-68.973	-107.974	
		3.000			-1904.433			.000
145	1	.000			-570.357			.000
		.000	38.952		-54.881	.502	-.097	
		3.000	38.952		61.976	.502	1.410	
		3.000			-570.357			.000
146	1	.000			-1220.361			.000
		.000	35.518		-49.342	-56.983	80.532	
		3.000	35.518		57.211	-56.983	-90.418	
		3.000			-1220.361			.000
147	1	.000			-1268.002			.000

		.000	35.044	-48.689	-89.450	125.752	
		3.000	35.044	56.443	-89.450	-142.599	
		3.000		-1268.002			.000
148		-----					
	1	.000		-909.113			.000
		.000	37.803	-53.305	-66.854	93.117	
		3.000	37.803	60.104	-66.854	-107.444	
		3.000		-909.113			.000
149		-----					
	1	.000		-1361.603			.000
		.000	145.397	-204.365	7.229	-9.866	
		3.000	145.397	231.826	7.229	11.820	
		3.000		-1361.603			.000
150		-----					
	1	.000		-3331.781			.000
		.000	151.914	-195.616	-68.167	87.163	
		3.000	151.914	260.126	-68.167	-117.338	
		3.000		-3331.781			.000
151		-----					
	1	.000		-3453.788			.000
		.000	151.914	-195.616	-90.585	117.733	
		3.000	151.914	260.126	-90.585	-154.022	
		3.000		-3453.788			.000
152		-----					
	1	.000		-1736.541			.000
		.000	145.397	-204.365	-78.117	108.796	
		3.000	145.397	231.826	-78.117	-125.557	
		3.000		-1736.541			.000
153		-----					
	1	.000		-1286.731			.000
		.000	103.287	-143.772	3.033	-3.580	
		3.000	103.287	166.090	3.033	5.520	
		3.000		-1286.731			.000
154		-----					
	1	.000		-2023.702			.000
		.000	114.611	-159.396	-54.583	77.212	
		3.000	114.611	184.436	-54.583	-86.537	
		3.000		-2023.702			.000
155		-----					
	1	.000		-2071.342			.000
		.000	115.085	-160.050	-91.850	129.072	
		3.000	115.085	185.205	-91.850	-146.479	
		3.000		-2071.342			.000
156		-----					
	1	.000		-1625.487			.000
		.000	104.437	-145.349	-69.384	96.600	
		3.000	104.437	167.962	-69.384	-111.553	
		3.000		-1625.487			.000
157		-----					
	1	.000		-548.660			.000
		.000	44.417	-65.629	-10.114	16.642	
		3.000	44.417	67.622	-10.114	-13.701	
		3.000		-548.660			.000
158		-----					
	1	.000		-1107.184			.000
		.000	43.199	-63.839	-46.635	67.237	
		3.000	43.199	65.757	-46.635	-72.668	
		3.000		-1107.184			.000
159		-----					
	1	.000		-1147.862			.000
		.000	42.828	-63.349	-67.487	96.161	
		3.000	42.828	65.134	-67.487	-106.299	
		3.000		-1147.862			.000
160		-----					
	1	.000		-808.108			.000
		.000	43.574	-64.493	-56.713	80.141	
		3.000	43.574	66.229	-56.713	-89.997	

161		3.000		-808.108			.000
	1	.000		-1211.005			.000
		.000	110.796	-158.028	-5.903	10.920	
		3.000	110.796	174.361	-5.903	-6.791	
		3.000		-1211.005			.000
162							
	1	.000		-2927.224			.000
		.000	141.067	-196.565	-69.946	98.773	
		3.000	141.067	226.636	-69.946	-111.066	
		3.000		-2927.224			.000
163							
	1	.000		-3033.114			.000
		.000	141.067	-196.565	-82.753	115.234	
		3.000	141.067	226.636	-82.753	-133.025	
		3.000		-3033.114			.000
164							
	1	.000		-1501.311			.000
		.000	110.796	-158.028	-64.764	90.975	
		3.000	110.796	174.361	-64.764	-103.318	
		3.000		-1501.311			.000
165							
	1	.000		-1099.862			.000
		.000	89.464	-126.863	-8.316	14.202	
		3.000	89.464	141.530	-8.316	-10.746	
		3.000		-1099.862			.000
166							
	1	.000		-1733.569			.000
		.000	97.804	-138.049	-45.011	65.008	
		3.000	97.804	155.364	-45.011	-70.024	
		3.000		-1733.569			.000
167							
	1	.000		-1774.247			.000
		.000	98.175	-138.539	-69.111	98.389	
		3.000	98.175	155.987	-69.111	-108.944	
		3.000		-1774.247			.000
168							
	1	.000		-1359.310			.000
		.000	90.307	-127.999	-58.511	82.581	
		3.000	90.307	142.923	-58.511	-92.952	
		3.000		-1359.310			.000
169							
	1	.000		-512.542			.000
		.000	30.112	-38.177	.218	-4.520	
		3.000	30.112	52.161	.218	-3.867	
		3.000		-512.542			.000
170							
	1	.000		-981.238			.000
		.000	27.773	-35.222	-39.498	57.554	
		3.000	27.773	48.097	-39.498	-60.941	
		3.000		-981.238			.000
171							
	1	.000		-1017.950			.000
		.000	27.283	-34.483	-65.168	95.869	
		3.000	27.283	47.367	-65.168	-99.634	
		3.000		-1017.950			.000
172							
	1	.000		-703.228			.000
		.000	29.071	-36.591	-55.345	80.492	
		3.000	29.071	50.623	-55.345	-85.544	
		3.000		-703.228			.000
173							
	1	.000		-1055.124			.000
		.000	102.160	-150.427	5.682	-12.649	
		3.000	102.160	156.054	5.682	4.397	
		3.000		-1055.124			.000
174							

	1	.000		-2518.062			.000
		.000	126.336	-174.611	-58.502	80.158	
		3.000	126.336	204.398	-58.502	-95.348	
		3.000		-2518.062			.000
175	-----						
	1	.000		-2609.322			.000
		.000	126.336	-174.611	-77.496	108.036	
		3.000	126.336	204.398	-77.496	-124.452	
		3.000		-2609.322			.000
176	-----						
	1	.000		-1270.624			.000
		.000	102.160	-150.427	-64.632	95.014	
		3.000	102.160	156.054	-64.632	-98.882	
		3.000		-1270.624			.000
177	-----						
	1	.000		-922.154			.000
		.000	83.839	-120.524	2.442	-7.889	
		3.000	83.839	130.994	2.442	-.563	
		3.000		-922.154			.000
178	-----						
	1	.000		-1452.154			.000
		.000	92.909	-135.067	-37.491	54.537	
		3.000	92.909	143.660	-37.491	-57.936	
		3.000		-1452.154			.000
179	-----						
	1	.000		-1488.866			.000
		.000	93.399	-135.806	-67.175	98.885	
		3.000	93.399	144.390	-67.175	-102.638	
		3.000		-1488.866			.000
180	-----						
	1	.000		-1112.840			.000
		.000	84.880	-122.109	-57.570	83.862	
		3.000	84.880	132.532	-57.570	-88.848	
		3.000		-1112.840			.000
181	-----						
	1	.000		-458.453			.000
		.000	16.217	-23.411	10.765	-14.398	
		3.000	16.217	25.240	10.765	17.896	
		3.000		-458.453			.000
182	-----						
	1	.000		-845.674			.000
		.000	13.382	-18.865	-31.979	45.620	
		3.000	13.382	21.280	-31.979	-50.317	
		3.000		-845.674			.000
183	-----						
	1	.000		-875.273			.000
		.000	12.729	-17.976	-62.776	89.298	
		3.000	12.729	20.211	-62.776	-99.029	
		3.000		-875.273			.000
184	-----						
	1	.000		-589.718			.000
		.000	14.911	-21.615	-56.384	78.543	
		3.000	14.911	23.118	-56.384	-90.609	
		3.000		-589.718			.000
185	-----						
	1	.000		-895.113			.000
		.000	92.588	-131.507	17.519	-23.916	
		3.000	92.588	146.257	17.519	28.640	
		3.000		-895.113			.000
186	-----						
	1	.000		-2117.230			.000
		.000	98.872	-140.917	-42.055	60.058	
		3.000	98.872	155.699	-42.055	-66.108	
		3.000		-2117.230			.000
187	-----						
	1	.000		-2194.234			.000
		.000	98.872	-140.917	-65.693	93.604	

		3.000	98.872	155.699	-65.693	-103.476	
188		3.000		-2194.234			.000
	1	.000		-1047.470			.000
		.000	92.588	-131.507	-67.823	93.974	
		3.000	92.588	146.257	-67.823	-109.496	
		3.000		-1047.470			.000
189							
	1	.000		-743.242			.000
		.000	81.486	-113.466	13.527	-18.225	
		3.000	81.486	130.992	13.527	22.355	
		3.000		-743.242			.000
190							
	1	.000		-1183.121			.000
		.000	92.406	-127.931	-29.536	42.191	
		3.000	92.406	149.287	-29.536	-46.416	
		3.000		-1183.121			.000
191							
	1	.000		-1212.719			.000
		.000	93.058	-128.819	-65.219	92.727	
		3.000	93.058	150.355	-65.219	-102.930	
		3.000		-1212.719			.000
192							
	1	.000		-874.506			.000
		.000	82.792	-115.261	-59.146	82.370	
		3.000	82.792	133.115	-59.146	-95.068	
		3.000		-874.506			.000
193							
	1	.000		-393.043			.000
		.000	21.448	-32.494	-2.095	4.568	
		3.000	21.448	31.850	-2.095	-1.716	
		3.000		-393.043			.000
194							
	1	.000		-700.172			.000
		.000	21.463	-32.128	-22.901	33.484	
		3.000	21.463	32.261	-22.901	-35.220	
		3.000		-700.172			.000
195							
	1	.000		-724.950			.000
		.000	21.100	-31.634	-36.369	52.680	
		3.000	21.100	31.667	-36.369	-56.427	
		3.000		-724.950			.000
196							
	1	.000		-475.192			.000
		.000	20.739	-31.526	-37.383	52.981	
		3.000	20.739	30.691	-37.383	-59.169	
		3.000		-475.192			.000
197							
	1	.000		-729.955			.000
		.000	56.160	-81.461	.891	.307	
		3.000	56.160	87.018	.891	2.980	
		3.000		-729.955			.000
198							
	1	.000		-1709.554			.000
		.000	104.024	-152.666	-52.788	77.274	
		3.000	104.024	159.406	-52.788	-81.092	
		3.000		-1709.554			.000
199							
	1	.000		-1770.485			.000
		.000	104.024	-152.666	-64.865	95.058	
		3.000	104.024	159.406	-64.865	-99.537	
		3.000		-1770.485			.000
200							
	1	.000		-826.930			.000
		.000	56.160	-81.461	-43.805	61.760	
		3.000	56.160	87.018	-43.805	-69.657	
		3.000		-826.930			.000

201	1	.000			-575.354		.000
		.000	56.276	-80.161		-.662	2.596
		3.000	56.276	88.668		-.662	.611
		3.000			-575.354		.000

202	1	.000			-920.743		.000
		.000	62.888	-89.099		-21.748	31.866
		3.000	62.888	99.566		-21.748	-33.378
		3.000			-920.743		.000

203	1	.000			-945.521		.000
		.000	63.251	-89.593		-37.522	54.298
		3.000	63.251	100.159		-37.522	-58.269
		3.000			-945.521		.000

204	1	.000			-657.504		.000
		.000	56.985	-81.128		-38.816	54.953
		3.000	56.985	89.827		-38.816	-61.496
		3.000			-657.504		.000

205	1	.000			-317.313		.000
		.000	8.220	-9.949		7.346	-12.312
		3.000	8.220	14.710		7.346	9.726
		3.000			-317.313		.000

206	1	.000			-542.901		.000
		.000	8.158	-10.284		-16.343	24.029
		3.000	8.158	14.190		-16.343	-24.999
		3.000			-542.901		.000

207	1	.000			-565.742		.000
		.000	7.706	-9.603		-32.317	47.703
		3.000	7.706	13.515		-32.317	-49.247
		3.000			-565.742		.000

208	1	.000			-362.825		.000
		.000	7.330	-8.612		-36.647	53.710
		3.000	7.330	13.378		-36.647	-56.231
		3.000			-362.825		.000

209	1	.000			-557.372		.000
		.000	46.165	-68.300		11.004	-17.670
		3.000	46.165	70.194		11.004	15.342
		3.000			-557.372		.000

210	1	.000			-1285.661		.000
		.000	74.697	-102.830		-36.585	50.374
		3.000	74.697	121.262		-36.585	-59.380
		3.000			-1285.661		.000

211	1	.000			-1327.779		.000
		.000	74.697	-102.830		-50.154	70.270
		3.000	74.697	121.262		-50.154	-80.192
		3.000			-1327.779		.000

212	1	.000			-608.940		.000
		.000	46.165	-68.300		-44.103	65.215
		3.000	46.165	70.194		-44.103	-67.094
		3.000			-608.940		.000

213	1	.000			-424.653		.000
		.000	51.853	-75.435		9.126	-14.980
		3.000	51.853	80.125		9.126	12.396
		3.000			-424.653		.000

214	1	.000			-667.929		.000

		.000	59.149	-87.006	-14.961	21.962		
		3.000	59.149	90.442	-14.961	-22.920		
		3.000		-667.929			.000	
215		-----						
	1	.000		-690.770			.000	
		.000	59.601	-87.687	-33.699	49.770		
		3.000	59.601	91.117	-33.699	-51.327		
		3.000		-690.770			.000	
216		-----						
	1	.000		-470.166			.000	
		.000	52.743	-76.772	-38.427	56.378		
		3.000	52.743	81.458	-38.427	-58.902		
		3.000		-470.166			.000	
217		-----						
	1	.000		-224.067			.000	
		.000	-.710	3.753	11.854	-19.351		
		3.000	-.710	1.623	11.854	16.211		
		3.000		-224.067			.000	
218		-----						
	1	.000		-374.100			.000	
		.000	-2.100	5.581	-8.308	11.903		
		3.000	-2.100	-.719	-8.308	-13.022		
		3.000		-374.100			.000	
219		-----						
	1	.000		-391.681			.000	
		.000	-2.529	6.233	-25.473	37.323		
		3.000	-2.529	-1.354	-25.473	-39.097		
		3.000		-391.681			.000	
220		-----						
	1	.000		-243.335			.000	
		.000	-1.574	5.060	-31.251	45.732		
		3.000	-1.574	.339	-31.251	-48.020		
		3.000		-243.335			.000	
221		-----						
	1	.000		-379.584			.000	
		.000	31.551	-46.136	15.882	-25.331		
		3.000	31.551	48.518	15.882	22.315		
		3.000		-379.584			.000	
222		-----						
	1	.000		-864.359			.000	
		.000	51.611	-68.245	-22.443	29.236		
		3.000	51.611	86.589	-22.443	-38.092		
		3.000		-864.359			.000	
223		-----						
	1	.000		-890.255			.000	
		.000	51.611	-68.245	-38.116	52.094		
		3.000	51.611	86.589	-38.116	-62.255		
		3.000		-890.255			.000	
224		-----						
	1	.000		-399.814			.000	
		.000	31.551	-46.136	-37.613	55.518		
		3.000	31.551	48.518	-37.613	-57.321		
		3.000		-399.814			.000	
225		-----						
	1	.000		-275.012			.000	
		.000	42.117	-60.868	13.605	-21.994		
		3.000	42.117	65.482	13.605	18.822		
		3.000		-275.012			.000	
226		-----						
	1	.000		-430.317			.000	
		.000	47.391	-69.194	-6.912	9.809		
		3.000	47.391	72.978	-6.912	-10.927		
		3.000		-430.317			.000	
227		-----						
	1	.000		-447.898			.000	
		.000	47.820	-69.846	-26.869	39.416		
		3.000	47.820	73.614	-26.869	-41.192		

228		3.000			-447.898			.000
	1	.000			-294.280			.000
		.000	42.980	-62.174		-33.002	48.375	
		3.000	42.980	66.767		-33.002	-50.632	
		3.000			-294.280			.000
229								
	1	.000			-112.164			.000
		.000	-16.093	23.554		23.064	-32.616	
		3.000	-16.093	-24.723		23.064	36.578	
		3.000			-112.164			.000
230								
	1	.000			-192.937			.000
		.000	-19.216	27.603		-.556	.022	
		3.000	-19.216	-30.046		-.556	-1.647	
		3.000			-192.937			.000
231								
	1	.000			-202.115			.000
		.000	-19.727	28.318		-18.612	26.974	
		3.000	-19.727	-30.862		-18.612	-28.862	
		3.000			-202.115			.000
232								
	1	.000			-116.373			.000
		.000	-17.151	25.031		-31.114	42.517	
		3.000	-17.151	-26.423		-31.114	-50.825	
		3.000			-116.373			.000
233								
	1	.000			-196.225			.000
		.000	17.030	-24.188		28.662	-40.265	
		3.000	17.030	26.900		28.662	45.721	
		3.000			-196.225			.000
234								
	1	.000			-446.343			.000
		.000	25.109	-29.107		-8.863	7.920	
		3.000	25.109	46.220		-8.863	-18.669	
		3.000			-446.343			.000
235								
	1	.000			-457.817			.000
		.000	25.109	-29.107		-24.066	31.626	
		3.000	25.109	46.220		-24.066	-40.572	
		3.000			-457.817			.000
236								
	1	.000			-199.832			.000
		.000	17.030	-24.188		-38.025	52.357	
		3.000	17.030	26.900		-38.025	-61.717	
		3.000			-199.832			.000
237								
	1	.000			-126.919			.000
		.000	37.827	-51.224		25.166	-35.571	
		3.000	37.827	62.255		25.166	39.926	
		3.000			-126.919			.000
238								
	1	.000			-208.160			.000
		.000	42.457	-58.086		.989	-2.220	
		3.000	42.457	69.284		.989	.747	
		3.000			-208.160			.000
239								
	1	.000			-217.338			.000
		.000	42.967	-58.802		-20.157	29.215	
		3.000	42.967	70.100		-20.157	-31.256	
		3.000			-217.338			.000
240								
	1	.000			-131.127			.000
		.000	38.885	-52.701		-33.215	45.472	
		3.000	38.885	63.955		-33.215	-54.173	
		3.000			-131.127			.000

PROGRAM:SAP90/FILE:\1b.F3F

15 storey building - Bay size 5*5 // Fc=25MPa

FRAME ELEMENT FORCES

ELT	LOAD	DIST	1-2 PLANE		AXIAL	1-3 PLANE		AXIAL
			SHEAR	MOMENT		FORCE	SHEAR	
ID	COMB	ENDI						
1								
	1	.000			-808.879			.000
		.000	-8.702	-9.957		9.338	-.750	
		3.000	-8.702	-36.065		9.338	27.263	
		3.000			-808.879			.000
2								
	1	.000			-2310.630			.000
		.000	-16.896	-2.275		-.643	8.608	
		3.000	-16.896	-52.963		-.643	6.679	
		3.000			-2310.630			.000
3								
	1	.000			-2353.976			.000
		.000	-16.920	-2.253		-1.897	9.783	
		3.000	-16.920	-53.012		-1.897	4.093	
		3.000			-2353.976			.000
4								
	1	.000			-1534.919			.000
		.000	-8.747	-9.915		-10.080	17.456	
		3.000	-8.747	-36.157		-10.080	-12.783	
		3.000			-1534.919			.000
5								
	1	.000			-2614.115			.000
		.000	2.555	-20.513		17.539	-8.440	
		3.000	2.555	-12.847		17.539	44.178	
		3.000			-2614.115			.000
6								
	1	.000			-6419.218			.000
		.000	.966	-58.117		-.585	25.820	
		3.000	.966	-55.219		-.585	24.064	
		3.000			-6419.218			.000
7								
	1	.000			-6549.571			.000
		.000	.966	-58.117		-.513	25.756	
		3.000	.966	-55.219		-.513	24.216	
		3.000			-6549.571			.000
8								
	1	.000			-3396.071			.000
		.000	2.555	-20.513		-18.314	25.176	
		3.000	2.555	-12.847		-18.314	-29.766	
		3.000			-3396.071			.000
9								
	1	.000			-2259.134			.000
		.000	10.613	-28.068		9.431	-.838	
		3.000	10.613	3.772		9.431	27.456	
		3.000			-2259.134			.000
10								
	1	.000			-3880.827			.000
		.000	18.821	-35.764		-.552	8.522	
		3.000	18.821	20.700		-.552	6.867	
		3.000			-3880.827			.000
11								
	1	.000			-3924.173			.000
		.000	18.845	-35.786		-1.988	9.869	
		3.000	18.845	20.749		-1.988	3.905	
		3.000			-3924.173			.000
12								
	1	.000			-2985.174			.000
		.000	10.658	-28.110		-10.173	17.543	
		3.000	10.658	3.864		-10.173	-12.976	
		3.000			-2985.174			.000
13								
	1	.000			-703.949			.000

		.000	-16.199	.795	17.724	-16.077	
		3.000	-16.199	-47.802	17.724	37.094	
		3.000		-703.949			.000
14		-----					
	1	.000		-2100.680			.000
		.000	-30.277	23.210	-1.411	13.090	
		3.000	-30.277	-67.620	-1.411	8.856	
		3.000		-2100.680			.000
15		-----					
	1	.000		-2142.934			.000
		.000	-30.348	23.304	-5.762	18.579	
		3.000	-30.348	-67.741	-5.762	1.293	
		3.000		-2142.934			.000
16		-----					
	1	.000		-1426.180			.000
		.000	-16.332	.970	-18.745	39.956	
		3.000	-16.332	-48.027	-18.745	-16.278	
		3.000		-1426.180			.000
17		-----					
	1	.000		-2402.099			.000
		.000	7.462	-34.053	31.879	-38.574	
		3.000	7.462	-11.666	31.879	57.062	
		3.000		-2402.099			.000
18		-----					
	1	.000		-5984.405			.000
		.000	-.316	-77.327	1.006	34.723	
		3.000	-.316	-78.275	1.006	37.741	
		3.000		-5984.405			.000
19		-----					
	1	.000		-6113.702			.000
		.000	-.316	-77.327	-.377	35.865	
		3.000	-.316	-78.275	-.377	34.734	
		3.000		-6113.702			.000
20		-----					
	1	.000		-3180.423			.000
		.000	7.462	-34.053	-32.993	62.577	
		3.000	7.462	-11.666	-32.993	-36.402	
		3.000		-3180.423			.000
21		-----					
	1	.000		-2145.374			.000
		.000	19.991	-54.863	18.003	-16.444	
		3.000	19.991	5.109	18.003	37.564	
		3.000		-2145.374			.000
22		-----					
	1	.000		-3662.319			.000
		.000	34.103	-77.327	-1.139	12.732	
		3.000	34.103	24.981	-1.139	9.315	
		3.000		-3662.319			.000
23		-----					
	1	.000		-3704.573			.000
		.000	34.174	-77.421	-6.034	18.937	
		3.000	34.174	25.102	-6.034	.835	
		3.000		-3704.573			.000
24		-----					
	1	.000		-2867.604			.000
		.000	20.124	-55.038	-19.024	40.323	
		3.000	20.124	5.334	-19.024	-16.748	
		3.000		-2867.604			.000
25		-----					
	1	.000		-604.189			.000
		.000	-15.093	-22.023	17.804	-4.278	
		3.000	-15.093	-67.301	17.804	49.134	
		3.000		-604.189			.000
26		-----					
	1	.000		-1898.697			.000
		.000	-26.868	-4.364	-3.549	26.039	
		3.000	-26.868	-84.968	-3.549	15.393	

	1	.000							
		.000	-16.231	-69.727	-19.130	78.471			.000
		3.000	-16.231	-118.421	-19.130	21.081			
		3.000		-1210.040					.000
41		-----							
	1	.000							
		.000	31.028	-134.758	31.172	2.167			.000
		3.000	31.028	-41.674	31.172	95.682			
		3.000		-1998.498					.000
42		-----							
	1	.000							
		.000	-15.318	-276.159	14.033	136.515			.000
		3.000	-15.318	-322.114	14.033	178.613			
		3.000		-5127.523					.000
43		-----							
	1	.000							
		.000	-15.318	-276.159	8.761	143.893			.000
		3.000	-15.318	-322.114	8.761	170.176			
		3.000		-5251.338					.000
44		-----							
	1	.000							
		.000	31.028	-134.758	-31.018	96.760			.000
		3.000	31.028	-41.674	-31.018	3.707			
		3.000		-2744.546					.000
45		-----							
	1	.000							
		.000	22.436	-128.129	20.482	18.712			.000
		3.000	22.436	-60.821	20.482	80.159			
		3.000		-1891.728					.000
46		-----							
	1	.000							
		.000	34.106	-146.113	-8.915	60.005			.000
		3.000	34.106	-43.794	-8.915	33.260			
		3.000		-3203.234					.000
47		-----							
	1	.000							
		.000	34.272	-146.353	-20.152	76.269			.000
		3.000	34.272	-43.538	-20.152	15.814			
		3.000		-3240.456					.000
48		-----							
	1	.000							
		.000	22.738	-128.567	-19.763	79.386			.000
		3.000	22.738	-60.352	-19.763	20.099			
		3.000		-2581.424					.000
49		-----							
	1	.000							
		.000	68.850	-190.933	-24.197	84.485			.000
		3.000	68.850	15.618	-24.197	11.894			
		3.000		-477.955					.000
50		-----							
	1	.000							
		.000	58.204	-175.299	-74.055	150.559			.000
		3.000	58.204	-.688	-74.055	-71.605			
		3.000		-1555.392					.000
51		-----							
	1	.000							
		.000	57.950	-174.959	-88.391	170.149			.000
		3.000	57.950	-1.111	-88.391	-95.023			
		3.000		-1589.558					.000
52		-----							
	1	.000							
		.000	68.396	-190.321	-72.015	150.867			.000
		3.000	68.396	14.865	-72.015	-65.177			
		3.000		-1113.851					.000
53		-----							
	1	.000							
		.000	154.145	-303.158	-12.375	67.511			.000

		3.000	154.145	159.279	-12.375	30.386	
		3.000		-1819.305			.000
54	-----						
	1	.000		-4704.437			.000
		.000	203.364	-597.316	-105.608	314.485	
		3.000	203.364	12.777	-105.608	-2.341	
		3.000		-4704.437			.000
55	-----						
	1	.000		-4823.870			.000
		.000	203.364	-597.316	-113.384	325.147	
		3.000	203.364	12.777	-113.384	-15.003	
		3.000		-4823.870			.000
56	-----						
	1	.000		-2510.775			.000
		.000	154.145	-303.158	-85.953	170.164	
		3.000	154.145	159.279	-85.953	-87.695	
		3.000		-2510.775			.000
57	-----						
	1	.000		-1742.380			.000
		.000	115.803	-256.116	-23.249	83.207	
		3.000	115.803	91.292	-23.249	13.459	
		3.000		-1742.380			.000
58	-----						
	1	.000		-2938.470			.000
		.000	129.939	-275.455	-73.132	149.313	
		3.000	129.939	114.363	-73.132	-70.083	
		3.000		-2938.470			.000
59	-----						
	1	.000		-2972.636			.000
		.000	130.193	-275.795	-89.314	171.395	
		3.000	130.193	114.785	-89.314	-96.545	
		3.000		-2972.636			.000
60	-----						
	1	.000		-2378.275			.000
		.000	116.257	-256.727	-72.962	152.145	
		3.000	116.257	92.044	-72.962	-66.743	
		3.000		-2378.275			.000
61	-----						
	1	.000		-472.309			.000
		.000	65.064	-120.024	-24.855	49.890	
		3.000	65.064	75.167	-24.855	-24.677	
		3.000		-472.309			.000
62	-----						
	1	.000		-1427.783			.000
		.000	59.823	-111.961	-68.118	109.002	
		3.000	59.823	67.509	-68.118	-95.353	
		3.000		-1427.783			.000
63	-----						
	1	.000		-1460.112			.000
		.000	59.586	-111.646	-80.200	125.371	
		3.000	59.586	67.114	-80.200	-115.228	
		3.000		-1460.112			.000
64	-----						
	1	.000		-1027.223			.000
		.000	64.634	-119.455	-62.742	101.681	
		3.000	64.634	74.446	-62.742	-86.546	
		3.000		-1027.223			.000
65	-----						
	1	.000		-1656.979			.000
		.000	141.001	-223.384	-17.640	39.414	
		3.000	141.001	199.617	-17.640	-13.505	
		3.000		-1656.979			.000
66	-----						
	1	.000		-4283.265			.000
		.000	211.186	-407.382	-111.456	212.698	
		3.000	211.186	226.176	-111.456	-121.670	
		3.000		-4283.265			.000

67	-----					
	1	.000		-4396.053		.000
		.000	211.186	-407.382	-118.012	222.068
		3.000	211.186	226.176	-118.012	-131.969
		3.000		-4396.053		.000
68	-----					
	1	.000		-2263.574		.000
		.000	141.001	-223.384	-73.513	116.542
		3.000	141.001	199.617	-73.513	-103.996
		3.000		-2263.574		.000
69	-----					
	1	.000		-1579.280		.000
		.000	102.333	-170.943	-23.976	48.722
		3.000	102.333	136.057	-23.976	-23.205
		3.000		-1579.280		.000
70	-----					
	1	.000		-2645.678		.000
		.000	114.013	-186.850	-67.279	107.882
		3.000	114.013	155.189	-67.279	-93.955
		3.000		-2645.678		.000
71	-----					
	1	.000		-2678.007		.000
		.000	114.250	-187.166	-81.039	126.490
		3.000	114.250	155.584	-81.039	-116.626
		3.000		-2678.007		.000
72	-----					
	1	.000		-2134.193		.000
		.000	102.763	-171.511	-63.622	102.849
		3.000	102.763	136.778	-63.622	-88.017
		3.000		-2134.193		.000
73	-----					
	1	.000		-479.199		.000
		.000	56.625	-84.984	-17.204	25.036
		3.000	56.625	84.891	-17.204	-26.576
		3.000		-479.199		.000
74	-----					
	1	.000		-1314.019		.000
		.000	51.891	-77.810	-68.925	103.804
		3.000	51.891	77.862	-68.925	-102.971
		3.000		-1314.019		.000
75	-----					
	1	.000		-1346.009		.000
		.000	51.569	-77.334	-85.198	127.689
		3.000	51.569	77.373	-85.198	-127.905
		3.000		-1346.009		.000
76	-----					
	1	.000		-943.331		.000
		.000	56.026	-84.101	-63.911	95.343
		3.000	56.026	83.976	-63.911	-96.390
		3.000		-943.331		.000
77	-----					
	1	.000		-1503.706		.000
		.000	148.599	-224.777	-9.819	13.702
		3.000	148.599	221.021	-9.819	-15.754
		3.000		-1503.706		.000
78	-----					
	1	.000		-3861.666		.000
		.000	176.505	-263.168	-90.308	131.819
		3.000	176.505	266.346	-90.308	-139.106
		3.000		-3861.666		.000
79	-----					
	1	.000		-3965.149		.000
		.000	176.505	-263.168	-98.357	143.483
		3.000	176.505	266.346	-98.357	-151.588
		3.000		-3965.149		.000
80	-----					
	1	.000		-2012.329		.000

		.000	148.599	-224.777	-75.912	113.971		
		3.000	148.599	221.021	-75.912	-113.765		
		3.000		-2012.329			.000	
81		-----						
	1	.000		-1409.499			.000	
		.000	102.527	-154.133	-15.980	23.236	.000	
		3.000	102.527	153.448	-15.980	-24.705		
		3.000		-1409.499			.000	
82		-----						
	1	.000		-2340.741			.000	
		.000	115.629	-174.626	-67.763	102.097	.000	
		3.000	115.629	172.262	-67.763	-101.191		
		3.000		-2340.741			.000	
83		-----						
	1	.000		-2372.732			.000	
		.000	115.951	-175.102	-86.360	129.396	.000	
		3.000	115.951	172.752	-86.360	-129.685		
		3.000		-2372.732			.000	
84		-----						
	1	.000		-1873.631			.000	
		.000	103.127	-155.016	-65.135	97.143	.000	
		3.000	103.127	154.363	-65.135	-98.261		
		3.000		-1873.631			.000	
85		-----						
	1	.000		-481.229			.000	
		.000	50.268	-68.946	-13.310	16.420	.000	
		3.000	50.268	81.859	-13.310	-23.510		
		3.000		-481.229			.000	
86		-----						
	1	.000		-1200.758			.000	
		.000	46.050	-62.410	-62.572	90.463	.000	
		3.000	46.050	75.738	-62.572	-97.254		
		3.000		-1200.758			.000	
87		-----						
	1	.000		-1230.721			.000	
		.000	45.690	-61.885	-81.011	117.469	.000	
		3.000	45.690	75.185	-81.011	-125.564		
		3.000		-1230.721			.000	
88		-----						
	1	.000		-856.057			.000	
		.000	49.586	-67.952	-61.551	87.702	.000	
		3.000	49.586	80.806	-61.551	-96.952		
		3.000		-856.057			.000	
89		-----						
	1	.000		-1351.744			.000	
		.000	138.419	-201.418	-6.627	6.270	.000	
		3.000	138.419	213.840	-6.627	-13.611		
		3.000		-1351.744			.000	
90		-----						
	1	.000		-3442.514			.000	
		.000	161.848	-215.042	-82.185	106.969	.000	
		3.000	161.848	270.501	-82.185	-139.586		
		3.000		-3442.514			.000	
91		-----						
	1	.000		-3536.166			.000	
		.000	161.848	-215.042	-91.327	120.451	.000	
		3.000	161.848	270.501	-91.327	-153.531		
		3.000		-3536.166			.000	
92		-----						
	1	.000		-1764.991			.000	
		.000	138.419	-201.418	-72.320	103.898	.000	
		3.000	138.419	213.840	-72.320	-113.062		
		3.000		-1764.991			.000	
93		-----						
	1	.000		-1235.861			.000	
		.000	97.565	-138.838	-11.913	14.384	.000	
		3.000	97.565	153.858	-11.913	-21.355		

	1	.000			-1772.919			.000
		.000	101.011	-143.001		-76.446	110.401	
		3.000	101.011	160.031		-76.446	-118.936	
		3.000			-1772.919			.000
108	-----							
	1	.000			-1353.831			.000
		.000	91.577	-128.002		-59.649	84.379	
		3.000	91.577	146.731		-59.649	-94.567	
		3.000			-1353.831			.000
109	-----							
	1	.000			-448.927			.000
		.000	31.774	-40.093		-.084	-1.923	
		3.000	31.774	55.230		-.084	-2.175	
		3.000			-448.927			.000
110	-----							
	1	.000			-958.432			.000
		.000	27.857	-33.975		-50.371	69.214	
		3.000	27.857	49.596		-50.371	-81.899	
		3.000			-958.432			.000
111	-----							
	1	.000			-980.503			.000
		.000	27.365	-33.300		-75.504	104.275	
		3.000	27.365	48.796		-75.504	-122.236	
		3.000			-980.503			.000
112	-----							
	1	.000			-665.270			.000
		.000	30.828	-38.793		-59.696	80.821	
		3.000	30.828	53.692		-59.696	-98.266	
		3.000			-665.270			.000
113	-----							
	1	.000			-1042.474			.000
		.000	121.905	-167.768		6.585	-11.551	
		3.000	121.905	197.947		6.585	8.203	
		3.000			-1042.474			.000
114	-----							
	1	.000			-2608.946			.000
		.000	112.254	-141.168		-53.534	66.933	
		3.000	112.254	195.594		-53.534	-93.669	
		3.000			-2608.946			.000
115	-----							
	1	.000			-2681.993			.000
		.000	112.254	-141.168		-65.600	84.084	
		3.000	112.254	195.594		-65.600	-112.716	
		3.000			-2681.993			.000
116	-----							
	1	.000			-1288.280			.000
		.000	121.905	-167.768		-70.603	95.821	
		3.000	121.905	197.947		-70.603	-115.989	
		3.000			-1288.280			.000
117	-----							
	1	.000			-887.937			.000
		.000	90.139	-120.992		1.857	-4.601	
		3.000	90.139	149.425		1.857	.970	
		3.000			-887.937			.000
118	-----							
	1	.000			-1462.780			.000
		.000	101.225	-136.064		-48.532	66.666	
		3.000	101.225	167.610		-48.532	-78.930	
		3.000			-1462.780			.000
119	-----							
	1	.000			-1484.851			.000
		.000	101.716	-136.739		-77.343	106.824	
		3.000	101.716	168.409		-77.343	-125.205	
		3.000			-1484.851			.000
120	-----							
	1	.000			-1104.280			.000
		.000	91.085	-122.293		-61.636	83.498	

		3.000	91.085	150.963	-61.636	-101.411	
121		3.000		-1104.280			.000
	1	.000		-412.018			.000
		.000	27.900	-38.003	-4.305	6.054	
		3.000	27.900	45.697	-4.305	-6.863	
		3.000		-412.018			.000
122							
	1	.000		-826.713			.000
		.000	27.754	-37.089	-34.836	48.688	
		3.000	27.754	46.174	-34.836	-55.821	
		3.000		-826.713			.000
123							
	1	.000		-845.179			.000
		.000	27.403	-36.612	-50.749	70.833	
		3.000	27.403	45.597	-50.749	-81.415	
		3.000		-845.179			.000
124							
	1	.000		-562.460			.000
		.000	27.211	-37.074	-45.275	61.887	
		3.000	27.211	44.559	-45.275	-73.937	
		3.000		-562.460			.000
125							
	1	.000		-883.820			.000
		.000	81.203	-112.967	-1.170	1.367	
		3.000	81.203	130.643	-1.170	-2.142	
		3.000		-883.820			.000
126							
	1	.000		-2191.134			.000
		.000	133.728	-179.085	-71.696	96.452	
		3.000	133.728	222.098	-71.696	-118.637	
		3.000		-2191.134			.000
127							
	1	.000		-2252.060			.000
		.000	133.728	-179.085	-79.030	107.649	
		3.000	133.728	222.098	-79.030	-129.440	
		3.000		-2252.060			.000
128							
	1	.000		-1057.745			.000
		.000	81.203	-112.967	-52.871	72.258	
		3.000	81.203	130.643	-52.871	-86.356	
		3.000		-1057.745			.000
129							
	1	.000		-719.253			.000
		.000	68.443	-93.134	-2.937	4.196	
		3.000	68.443	112.196	-2.937	-4.616	
		3.000		-719.253			.000
130							
	1	.000		-1186.843			.000
		.000	76.719	-104.285	-33.600	46.991	
		3.000	76.719	125.873	-33.600	-53.810	
		3.000		-1186.843			.000
131							
	1	.000		-1205.309			.000
		.000	77.071	-104.761	-51.985	72.530	
		3.000	77.071	126.450	-51.985	-83.425	
		3.000		-1205.309			.000
132							
	1	.000		-869.695			.000
		.000	69.132	-94.063	-46.643	63.745	
		3.000	69.132	113.334	-46.643	-76.184	
		3.000		-869.695			.000
133							
	1	.000		-356.883			.000
		.000	17.345	-18.934	3.504	-9.496	
		3.000	17.345	33.102	3.504	1.017	
		3.000		-356.883			.000

		.000	2.508	-1.140	-43.328	60.772	
		3.000	2.508	6.385	-43.328	-69.212	
		3.000			-544.426		.000
148	-----						
	1	.000			-339.821		.000
		.000	1.773	-.619	-44.566	60.647	
		3.000	1.773	4.700	-44.566	-73.050	
		3.000			-339.821		.000
149	-----						
	1	.000			-553.661		.000
		.000	60.555	-84.256	17.392	-25.250	
		3.000	60.555	97.408	17.392	26.926	
		3.000			-553.661		.000
150	-----						
	1	.000			-1343.832		.000
		.000	69.660	-74.943	-35.932	37.566	
		3.000	69.660	134.036	-35.932	-70.231	
		3.000			-1343.832		.000
151	-----						
	1	.000			-1376.388		.000
		.000	69.660	-74.943	-44.542	50.691	
		3.000	69.660	134.036	-44.542	-82.935	
		3.000			-1376.388		.000
152	-----						
	1	.000			-618.369		.000
		.000	60.555	-84.256	-52.496	71.666	
		3.000	60.555	97.408	-52.496	-85.824	
		3.000			-618.369		.000
153	-----						
	1	.000			-401.183		.000
		.000	61.106	-82.468	15.995	-23.115	
		3.000	61.106	100.849	15.995	24.871	
		3.000			-401.183		.000
154	-----						
	1	.000			-672.114		.000
		.000	68.717	-92.987	-18.397	25.212	
		3.000	68.717	113.164	-18.397	-29.977	
		3.000			-672.114		.000
155	-----						
	1	.000			-684.151		.000
		.000	69.219	-93.682	-45.120	63.299	
		3.000	69.219	113.974	-45.120	-72.061	
		3.000			-684.151		.000
156	-----						
	1	.000			-456.797		.000
		.000	62.137	-83.891	-46.597	63.472	
		3.000	62.137	102.520	-46.597	-76.320	
		3.000			-456.797		.000
157	-----						
	1	.000			-200.118		.000
		.000	7.507	-10.417	3.082	-4.243	
		3.000	7.507	12.104	3.082	5.003	
		3.000			-200.118		.000
158	-----						
	1	.000			-369.673		.000
		.000	7.740	-10.844	-12.443	17.755	
		3.000	7.740	12.377	-12.443	-19.575	
		3.000			-369.673		.000
159	-----						
	1	.000			-378.233		.000
		.000	7.487	-10.493	-22.383	31.934	
		3.000	7.487	11.969	-22.383	-35.216	
		3.000			-378.233		.000
160	-----						
	1	.000			-223.950		.000
		.000	6.985	-9.692	-26.358	36.680	
		3.000	6.985	11.264	-26.358	-42.393	

PROGRAM:SAP90/FILE:\lc.F3F

10 storey building - Bay size 5*5 // Fc=25MPa

FRAME ELEMENT FORCES

ELT ID	LOAD COMB	DIST ENDD	1-2 PLANE			1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT	AXIAL FORCE	SHEAR	MOMENT	

1								
	1	.000			-734.353			.000
		.000	-7.682	.127		8.522	-4.776	
		3.000	-7.682	-22.919		8.522	20.789	
		3.000			-734.353			.000

2								
	1	.000			-1749.249			.000
		.000	-14.991	7.119		-1.036	4.368	
		3.000	-14.991	-37.853		-1.036	1.260	
		3.000			-1749.249			.000

3								
	1	.000			-1750.932			.000
		.000	-15.011	7.139		-1.843	5.140	
		3.000	-15.011	-37.894		-1.843	-.389	
		3.000			-1750.932			.000

4								
	1	.000			-1046.932			.000
		.000	-7.714	.158		-10.011	12.954	
		3.000	-7.714	-22.985		-10.011	-17.079	
		3.000			-1046.932			.000

5								
	1	.000			-1866.318			.000
		.000	2.789	-9.890		15.839	-11.776	
		3.000	2.789	-1.523		15.839	35.742	
		3.000			-1866.318			.000

6								
	1	.000			-4365.359			.000
		.000	3.553	-30.995		-2.773	15.509	
		3.000	3.553	-20.336		-2.773	7.189	
		3.000			-4365.359			.000

7								
	1	.000			-4404.751			.000
		.000	3.553	-30.995		-1.158	14.032	
		3.000	3.553	-20.336		-1.158	10.556	
		3.000			-4404.751			.000

8								
	1	.000			-2208.552			.000
		.000	2.789	-9.890		-17.396	20.019	
		3.000	2.789	-1.523		-17.396	-32.169	
		3.000			-2208.552			.000

9								
	1	.000			-1356.751			.000
		.000	10.792	-17.546		8.588	-4.839	
		3.000	10.792	14.830		8.588	20.925	
		3.000			-1356.751			.000

10								
	1	.000			-2435.125			.000
		.000	18.193	-24.627		-.973	4.307	
		3.000	18.193	29.953		-.973	1.389	
		3.000			-2435.125			.000

11								
	1	.000			-2436.808			.000
		.000	18.213	-24.646		-1.906	5.200	
		3.000	18.213	29.994		-1.906	-.518	
		3.000			-2436.808			.000

12								
	1	.000			-1669.330			.000
		.000	10.825	-17.577		-10.077	13.017	
		3.000	10.825	14.896		-10.077	-17.215	
		3.000			-1669.330			.000

13								
	1	.000			-629.405			.000

		.000	-14.197	14.183	16.063	-21.093	
		3.000	-14.197	-28.407	16.063	27.096	
		3.000		-629.405			.000
14		-----					
	1	.000		-1539.662			.000
		.000	-26.415	33.815	-1.820	6.401	
		3.000	-26.415	-45.432	-1.820	.941	
		3.000		-1539.662			.000
15		-----					
	1	.000		-1540.664			.000
		.000	-26.475	33.894	-5.350	10.655	
		3.000	-26.475	-45.532	-5.350	-5.395	
		3.000		-1540.664			.000
16		-----					
	1	.000		-938.922			.000
		.000	-14.293	14.311	-18.893	32.487	
		3.000	-14.293	-28.570	-18.893	-24.191	
		3.000		-938.922			.000
17		-----					
	1	.000		-1654.879			.000
		.000	7.141	-17.608	28.419	-40.865	
		3.000	7.141	3.813	28.419	44.392	
		3.000		-1654.879			.000
18		-----					
	1	.000		-3924.075			.000
		.000	4.870	-38.371	-3.542	20.063	
		3.000	4.870	-23.762	-3.542	9.438	
		3.000		-3924.075			.000
19		-----					
	1	.000		-3962.661			.000
		.000	4.870	-38.371	-1.760	16.544	
		3.000	4.870	-23.762	-1.760	11.264	
		3.000		-3962.661			.000
20		-----					
	1	.000		-1994.000			.000
		.000	7.141	-17.608	-31.468	52.542	
		3.000	7.141	3.813	-31.468	-41.862	
		3.000		-1994.000			.000
21		-----					
	1	.000		-1245.018			.000
		.000	20.666	-39.244	16.261	-21.354	
		3.000	20.666	22.753	16.261	27.428	
		3.000		-1245.018			.000
22		-----					
	1	.000		-2218.595			.000
		.000	33.197	-59.275	-1.632	6.153	
		3.000	33.197	40.316	-1.632	1.257	
		3.000		-2218.595			.000
23		-----					
	1	.000		-2219.597			.000
		.000	33.257	-59.354	-5.538	10.904	
		3.000	33.257	40.417	-5.538	-5.711	
		3.000		-2219.597			.000
24		-----					
	1	.000		-1554.535			.000
		.000	20.762	-39.372	-19.091	32.749	
		3.000	20.762	22.915	-19.091	-24.523	
		3.000		-1554.535			.000
25		-----					
	1	.000		-526.047			.000
		.000	-12.659	3.758	15.649	-15.685	
		3.000	-12.659	-34.218	15.649	31.262	
		3.000		-526.047			.000
26		-----					
	1	.000		-1336.097			.000
		.000	-22.241	18.260	-3.079	11.279	
		3.000	-22.241	-48.462	-3.079	2.041	

	1	.000							
		.000							
		3.000	-11.024	-20.373	-21.331		51.628		.000
		3.000	-11.024	-53.446	-21.331		-12.365		
		3.000							
41									
	1	.000							
		.000							
		3.000	26.618	-72.656	24.433		-18.106		.000
		3.000	26.618	7.197	24.433		55.192		
		3.000							
42									
	1	.000							
		.000							
		3.000	-8.426	-140.524	6.304		70.604		.000
		3.000	-8.426	-165.802	6.304		89.517		
		3.000							
43									
	1	.000							
		.000							
		3.000	-8.426	-140.524	5.902		70.726		.000
		3.000	-8.426	-165.802	5.902		88.430		
		3.000							
44									
	1	.000							
		.000							
		3.000	26.618	-72.656	-31.060		66.552		.000
		3.000	26.618	7.197	-31.060		-26.628		
		3.000							
45									
	1	.000							
		.000							
		3.000	26.087	-76.539	16.179		-5.099		.000
		3.000	26.087	1.722	16.179		43.437		
		3.000							
46									
	1	.000							
		.000							
		3.000	35.866	-91.435	-7.998		29.346		.000
		3.000	35.866	16.164	-7.998		5.352		
		3.000							
47									
	1	.000							
		.000							
		3.000	36.004	-91.635	-17.596		43.198		.000
		3.000	36.004	16.377	-17.596		-9.589		
		3.000							
48									
	1	.000							
		.000							
		3.000	26.303	-76.854	-21.774		52.272		.000
		3.000	26.303	2.056	-21.774		-13.051		
		3.000							
49									
	1	.000							
		.000							
		3.000	39.560	-95.711	-8.994		34.290		.000
		3.000	39.560	22.968	-8.994		7.309		
		3.000							
50									
	1	.000							
		.000							
		3.000	33.287	-86.086	-49.957		88.424		.000
		3.000	33.287	13.775	-49.957		-61.445		
		3.000							
51									
	1	.000							
		.000							
		3.000	33.074	-85.801	-62.449		105.630		.000
		3.000	33.074	13.422	-62.449		-81.716		
		3.000							
52									
	1	.000							
		.000							
		3.000	39.229	-95.268	-55.248		98.196		.000
		3.000	39.229	22.420	-55.248		-67.548		
		3.000							
53									
	1	.000							
		.000							
		3.000	109.023	-186.669	-.814		22.345		.000

		3.000	109.023	140.400				
		3.000			-1068.160	-.814	19.902	
54								.000
	1	.000			-2615.698			.000
		.000	172.180	-386.076		-91.315	204.890	.000
		3.000	172.180	130.465		-91.315	-69.056	
		3.000			-2615.698			.000
55								.000
	1	.000			-2645.811			.000
		.000	172.180	-386.076		-92.025	206.258	.000
		3.000	172.180	130.465		-92.025	-69.817	
		3.000			-2645.811			.000
56								.000
	1	.000			-1341.024			.000
		.000	109.023	-186.669		-66.652	114.061	.000
		3.000	109.023	140.400		-66.652	-85.894	
		3.000			-1341.024			.000
57								.000
	1	.000			-858.627			.000
		.000	85.659	-159.351		-8.323	33.386	.000
		3.000	85.659	97.628		-8.323	8.418	
		3.000			-858.627			.000
58								.000
	1	.000			-1520.299			.000
		.000	97.879	-176.127		-49.321	87.564	.000
		3.000	97.879	117.509		-49.321	-60.399	
		3.000			-1520.299			.000
59								.000
	1	.000			-1517.217			.000
		.000	98.092	-176.413		-63.084	106.490	.000
		3.000	98.092	117.862		-63.084	-82.763	
		3.000			-1517.217			.000
60								.000
	1	.000			-1105.470			.000
		.000	85.990	-159.794		-55.919	99.100	.000
		3.000	85.990	98.175		-55.919	-68.657	
		3.000			-1105.470			.000
61								.000
	1	.000			-339.744			.000
		.000	43.804	-70.491		-15.294	26.535	.000
		3.000	43.804	60.921		-15.294	-19.346	
		3.000			-339.744			.000
62								.000
	1	.000			-833.582			.000
		.000	41.308	-66.757		-45.454	67.970	.000
		3.000	41.308	57.168		-45.454	-68.392	
		3.000			-833.582			.000
63								.000
	1	.000			-832.416			.000
		.000	41.128	-66.520		-53.854	79.545	.000
		3.000	41.128	56.863		-53.854	-82.017	
		3.000			-832.416			.000
64								.000
	1	.000			-525.545			.000
		.000	43.534	-70.134		-47.638	70.520	.000
		3.000	43.534	60.469		-47.638	-72.395	
		3.000			-525.545			.000
65								.000
	1	.000			-899.317			.000
		.000	95.769	-141.895		-10.643	20.060	.000
		3.000	95.769	145.410		-10.643	-11.867	
		3.000			-899.317			.000
66								.000
	1	.000			-2179.354			.000
		.000	131.967	-202.194		-71.126	108.662	.000
		3.000	131.967	193.708		-71.126	-104.715	
		3.000			-2179.354			.000

67	-----						
	1	.000		-2204.535			.000
		.000	131.967	-202.194	-71.173	108.736	
		3.000	131.967	193.708	-71.173	-104.782	
		3.000		-2204.535			.000
68	-----						
	1	.000		-1105.348			.000
		.000	95.769	-141.895	-55.654	81.359	
		3.000	95.769	145.410	-55.654	-85.603	
		3.000		-1105.348			.000
69	-----						
	1	.000		-708.074			.000
		.000	76.232	-114.536	-14.765	25.830	
		3.000	76.232	114.161	-14.765	-18.465	
		3.000		-708.074			.000
70	-----						
	1	.000		-1245.802			.000
		.000	85.331	-126.738	-44.975	67.325	
		3.000	85.331	129.255	-44.975	-67.601	
		3.000		-1245.802			.000
71	-----						
	1	.000		-1244.636			.000
		.000	85.512	-126.975	-54.333	80.190	
		3.000	85.512	129.560	-54.333	-82.808	
		3.000		-1244.636			.000
72	-----						
	1	.000		-893.876			.000
		.000	76.502	-114.893	-48.167	71.225	
		3.000	76.502	114.612	-48.167	-73.276	
		3.000		-893.876			.000
73	-----						
	1	.000		-308.487			.000
		.000	29.126	-37.419	-5.110	4.168	
		3.000	29.126	49.958	-5.110	-11.161	
		3.000		-308.487			.000
74	-----						
	1	.000		-690.685			.000
		.000	26.088	-33.139	-39.487	57.315	
		3.000	26.088	45.125	-39.487	-61.147	
		3.000		-690.685			.000
75	-----						
	1	.000		-694.023			.000
		.000	25.837	-32.766	-50.687	73.662	
		3.000	25.837	44.746	-50.687	-78.399	
		3.000		-694.023			.000
76	-----						
	1	.000		-433.014			.000
		.000	28.760	-36.874	-45.434	64.976	
		3.000	28.760	49.406	-45.434	-71.326	
		3.000		-433.014			.000
77	-----						
	1	.000		-731.898			.000
		.000	88.293	-128.911	.563	-4.304	
		3.000	88.293	135.968	.563	-2.614	
		3.000		-731.898			.000
78	-----						
	1	.000		-1740.457			.000
		.000	109.667	-149.838	-57.773	79.060	
		3.000	109.667	179.164	-57.773	-94.260	
		3.000		-1740.457			.000
79	-----						
	1	.000		-1760.392			.000
		.000	109.667	-149.838	-59.429	80.666	
		3.000	109.667	179.164	-59.429	-97.621	
		3.000		-1760.392			.000
80	-----						
	1	.000		-869.001			.000

		.000	88.293	-128.911	-55.048	79.738	
		3.000	88.293	135.968	-55.048	-85.406	
		3.000					
81				-869.001			.000
	1	.000		-558.566			.000
		.000	69.657	-98.536	-4.396	3.107	
		3.000	69.657	110.434	-4.396	-10.081	
		3.000		-558.566			.000
82							
	1	.000		-968.415			.000
		.000	80.409	-115.239	-38.848	56.368	
		3.000	80.409	125.989	-38.848	-60.176	
		3.000		-968.415			.000
83							
	1	.000		-971.753			.000
		.000	80.660	-115.612	-51.326	74.609	
		3.000	80.660	126.368	-51.326	-79.370	
		3.000		-971.753			.000
84							
	1	.000		-683.094			.000
		.000	70.023	-99.082	-46.148	66.037	
		3.000	70.023	110.986	-46.148	-72.406	
		3.000		-683.094			.000
85							
	1	.000		-260.604			.000
		.000	18.722	-20.632	.650	-4.689	
		3.000	18.722	35.533	.650	-2.741	
		3.000		-260.604			.000
86							
	1	.000		-538.321			.000
		.000	14.726	-14.868	-29.552	41.236	
		3.000	14.726	29.310	-29.552	-47.420	
		3.000		-538.321			.000
87							
	1	.000		-543.568			.000
		.000	14.458	-14.472	-42.037	59.739	
		3.000	14.458	28.903	-42.037	-66.374	
		3.000		-543.568			.000
88							
	1	.000		-333.692			.000
		.000	18.331	-20.055	-40.770	56.541	
		3.000	18.331	34.937	-40.770	-65.770	
		3.000		-333.692			.000
89							
	1	.000		-560.127			.000
		.000	69.724	-98.703	6.733	-13.636	
		3.000	69.724	110.468	6.733	6.564	
		3.000		-560.127			.000
90							
	1	.000		-1306.338			.000
		.000	86.876	-110.459	-45.031	57.044	
		3.000	86.876	150.169	-45.031	-78.050	
		3.000		-1306.338			.000
91							
	1	.000		-1320.908			.000
		.000	86.876	-110.459	-48.338	61.984	
		3.000	86.876	150.169	-48.338	-83.030	
		3.000		-1320.908			.000
92							
	1	.000		-639.951			.000
		.000	69.724	-98.703	-49.774	70.027	
		3.000	69.724	110.468	-49.774	-79.294	
		3.000		-639.951			.000
93							
	1	.000		-409.890			.000
		.000	60.332	-82.157	1.415	-5.819	
		3.000	60.332	98.839	1.415	-1.575	

	1	.000			-457.520			.000	
		.000	55.949		-76.440		-32.822	45.548	
		3.000	55.949		91.407		-32.822	-52.918	
		3.000			-457.520			.000	
108	-----								
	1	.000			-300.043			.000	
		.000	48.047		-63.913		-34.424	47.782	
		3.000	48.047		80.228		-34.424	-55.490	
		3.000			-300.043			.000	
109	-----								
	1	.000			-101.641			.000	
		.000	-15.200		25.011		22.816	-32.373	
		3.000	-15.200		-20.590		22.816	36.077	
		3.000			-101.641			.000	
110	-----								
	1	.000			-193.079			.000	
		.000	-22.942		34.997		-6.694	6.567	
		3.000	-22.942		-33.830		-6.694	-13.514	
		3.000			-193.079			.000	
111	-----								
	1	.000			-195.905			.000	
		.000	-23.311		35.478		-22.695	29.691	
		3.000	-23.311		-34.455		-22.695	-38.393	
		3.000			-195.905			.000	
112	-----								
	1	.000			-111.928			.000	
		.000	-15.754		25.729		-35.703	43.338	
		3.000	-15.754		-21.532		-35.703	-63.772	
		3.000			-111.928			.000	
113	-----								
	1	.000			-196.452			.000	
		.000	28.266		-35.079		32.340	-44.604	
		3.000	28.266		49.718		32.340	52.417	
		3.000			-196.452			.000	
114	-----								
	1	.000			-446.480			.000	
		.000	30.489		-24.937		-15.514	10.706	
		3.000	30.489		66.531		-15.514	-35.836	
		3.000			-446.480			.000	
115	-----								
	1	.000			-450.934			.000	
		.000	30.489		-24.937		-18.720	18.200	
		3.000	30.489		66.531		-18.720	-37.960	
		3.000			-450.934			.000	
116	-----								
	1	.000			-207.596			.000	
		.000	28.266		-35.079		-46.776	57.768	
		3.000	28.266		49.718		-46.776	-82.560	
		3.000			-207.596			.000	
117	-----								
	1	.000			-124.401			.000	
		.000	43.889		-51.311		23.870	-33.764	
		3.000	43.889		80.355		23.870	37.845	
		3.000			-124.401			.000	
118	-----								
	1	.000			-218.258			.000	
		.000	54.102		-64.795		-5.800	5.346	
		3.000	54.102		97.511		-5.800	-12.053	
		3.000			-218.258			.000	
119	-----								
	1	.000			-221.084			.000	
		.000	54.470		-65.276		-23.589	30.912	
		3.000	54.470		98.135		-23.589	-39.854	
		3.000			-221.084			.000	
120	-----								
	1	.000			-134.688			.000	
		.000	44.442		-52.029		-36.756	44.729	

3.000	44.442	81.297	-36.756	-65.540	
3.000		-134.688			.000

	1	.000			-265.503			.000
		.000	27.353	-51.591		-13.536	27.136	
		3.000	27.353	30.468		-13.536	-13.473	
		3.000			-265.503			.000
14		-----						
	1	.000			-734.223			.000
		.000	17.893	-36.278		-42.403	69.296	
		3.000	17.893	17.400		-42.403	-57.913	
		3.000			-734.223			.000
15		-----						
	1	.000			-721.552			.000
		.000	17.795	-36.146		-43.390	69.935	
		3.000	17.795	17.239		-43.390	-60.233	
		3.000			-721.552			.000
16		-----						
	1	.000			-407.024			.000
		.000	27.253	-51.456		-43.831	73.534	
		3.000	27.253	30.303		-43.831	-57.959	
		3.000			-407.024			.000
17		-----						
	1	.000			-764.475			.000
		.000	65.377	-105.774		-3.148	10.625	
		3.000	65.377	90.357		-3.148	1.180	
		3.000			-764.475			.000
18		-----						
	1	.000			-1766.131			.000
		.000	96.714	-172.380		-67.342	119.766	
		3.000	96.714	117.763		-67.342	-82.261	
		3.000			-1766.131			.000
19		-----						
	1	.000			-1775.361			.000
		.000	96.714	-172.380		-64.666	114.376	
		3.000	96.714	117.763		-64.666	-79.623	
		3.000			-1775.361			.000
20		-----						
	1	.000			-924.770			.000
		.000	65.377	-105.774		-57.252	94.112	
		3.000	65.377	90.357		-57.252	-77.643	
		3.000			-924.770			.000
21		-----						
	1	.000			-480.558			.000
		.000	57.757	-98.036		-13.386	26.934	
		3.000	57.757	75.235		-13.386	-13.225	
		3.000			-480.558			.000
22		-----						
	1	.000			-980.390			.000
		.000	72.076	-119.861		-42.268	69.115	
		3.000	72.076	96.367		-42.268	-57.690	
		3.000			-980.390			.000
23		-----						
	1	.000			-967.719			.000
		.000	72.173	-119.992		-43.524	70.117	
		3.000	72.173	96.528		-43.524	-60.456	
		3.000			-967.719			.000
24		-----						
	1	.000			-622.078			.000
		.000	57.857	-98.170		-43.981	73.736	
		3.000	57.857	75.400		-43.981	-58.207	
		3.000			-622.078			.000
25		-----						
	1	.000			-220.488			.000
		.000	20.163	-27.409		-8.185	10.435	
		3.000	20.163	33.078		-8.185	-14.120	
		3.000			-220.488			.000
26		-----						
	1	.000			-562.493			.000
		.000	12.200	-15.797		-38.440	56.224	

		3.000	12.200	20.802	-38.440	-59.095	
		3.000		-562.493			.000
27	-----						
	1	.000		-555.969			.000
		.000	12.057	-15.592	-41.409	60.473	
		3.000	12.057	20.578	-41.409	-63.754	
		3.000		-555.969			.000
28	-----						
	1	.000		-309.938			.000
		.000	20.014	-27.197	-38.656	55.093	
		3.000	20.014	32.844	-38.656	-60.876	
		3.000		-309.938			.000
29	-----						
	1	.000		-581.640			.000
		.000	61.938	-90.945	.945	-2.994	
		3.000	61.938	94.868	.945	-.160	
		3.000		-581.640			.000
30	-----						
	1	.000		-1325.611			.000
		.000	79.312	-110.715	-53.583	74.384	
		3.000	79.312	127.223	-53.583	-86.364	
		3.000		-1325.611			.000
31	-----						
	1	.000		-1332.372			.000
		.000	79.312	-110.715	-53.062	73.808	
		3.000	79.312	127.223	-53.062	-85.378	
		3.000		-1332.372			.000
32	-----						
	1	.000		-682.648			.000
		.000	61.938	-90.945	-51.271	73.994	
		3.000	61.938	94.868	-51.271	-79.820	
		3.000		-682.648			.000
33	-----						
	1	.000		-357.666			.000
		.000	50.941	-72.540	-7.963	10.117	
		3.000	50.941	80.283	-7.963	-13.771	
		3.000		-357.666			.000
34	-----						
	1	.000		-718.960			.000
		.000	64.450	-92.884	-38.241	55.940	
		3.000	64.450	100.464	-38.241	-58.784	
		3.000		-718.960			.000
35	-----						
	1	.000		-712.436			.000
		.000	64.593	-93.090	-41.607	60.757	
		3.000	64.593	100.689	-41.607	-64.065	
		3.000		-712.436			.000
36	-----						
	1	.000		-447.116			.000
		.000	51.090	-72.752	-38.879	55.411	
		3.000	51.090	80.516	-38.879	-61.225	
		3.000		-447.116			.000
37	-----						
	1	.000		-165.395			.000
		.000	11.841	-9.773	-2.812	-1.433	
		3.000	11.841	25.749	-2.812	-9.867	
		3.000		-165.395			.000
38	-----						
	1	.000		-386.206			.000
		.000	3.691	2.685	-27.714	38.338	
		3.000	3.691	13.758	-27.714	-44.804	
		3.000		-386.206			.000
39	-----						
	1	.000		-383.877			.000
		.000	3.538	2.919	-32.246	44.517	
		3.000	3.538	13.533	-32.246	-52.220	
		3.000		-383.877			.000

		.000	28.041	-35.221	26.714	-37.655	
		3.000	28.041	48.901	26.714	42.488	
		3.000		-203.398			.000
54		-----					
	1	.000		-452.033			.000
		.000	31.524	-27.843	-22.604	18.863	
		3.000	31.524	66.728	-22.604	-48.950	
		3.000		-452.033			.000
55		-----					
	1	.000		-454.071			.000
		.000	31.524	-27.843	-20.176	19.113	
		3.000	31.524	66.728	-20.176	-41.416	
		3.000		-454.071			.000
56		-----					
	1	.000		-219.499			.000
		.000	28.041	-35.221	-46.940	57.617	
		3.000	28.041	48.901	-46.940	-83.202	
		3.000		-219.499			.000
57		-----					
	1	.000		-111.476			.000
		.000	37.172	-43.157	13.729	-20.797	
		3.000	37.172	68.358	13.729	20.391	
		3.000		-111.476			.000
58		-----					
	1	.000		-224.083			.000
		.000	52.788	-63.728	-15.340	18.196	
		3.000	52.788	94.635	-15.340	-27.825	
		3.000		-224.083			.000
59		-----					
	1	.000		-223.416			.000
		.000	53.022	-64.033	-20.472	26.435	
		3.000	53.022	95.034	-20.472	-34.981	
		3.000		-223.416			.000
60		-----					
	1	.000		-125.418			.000
		.000	37.422	-43.480	-31.908	37.859	
		3.000	37.422	68.787	-31.908	-57.866	
		3.000		-125.418			.000

PROGRAM:SAP90/FILE:\3d.F3F

5 storey building-Bay size 7*7 //Fc=25MPa

FRAME ELEMENT FORCES

ELT ID	LOAD COMB	DIST ENDI	1-2 PLANE		AXIAL FORCE	1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT		SHEAR	MOMENT	

1	1	.000			-898.294			.000
		.000	-24.106	-81.630		26.846	42.453	
		3.000	-24.106	-153.947		26.846	122.991	
		3.000			-898.294			.000

2	1	.000			-2235.927			.000
		.000	-50.113	-57.245		-14.144	80.886	
		3.000	-50.113	-207.584		-14.144	38.452	
		3.000			-2235.927			.000

3	1	.000			-2224.664			.000
		.000	-50.123	-57.235		-13.662	80.434	
		3.000	-50.123	-207.606		-13.662	39.447	
		3.000			-2224.664			.000

4	1	.000			-1166.618			.000
		.000	-24.116	-81.620		-29.518	95.300	
		3.000	-24.116	-153.969		-29.518	6.746	
		3.000			-1166.618			.000

5	1	.000			-2301.727			.000
		.000	23.729	-126.479		52.938	17.990	
		3.000	23.729	-55.293		52.938	176.803	
		3.000			-2301.727			.000

6	1	.000			-4947.254			.000
		.000	13.315	-205.285		-7.947	132.554	
		3.000	13.315	-165.340		-7.947	108.714	
		3.000			-4947.254			.000

7	1	.000			-4948.915			.000
		.000	13.315	-205.285		-4.969	129.829	
		3.000	13.315	-165.340		-4.969	114.923	
		3.000			-4948.915			.000

8	1	.000			-2582.242			.000
		.000	23.729	-126.479		-56.367	120.474	
		3.000	23.729	-55.293		-56.367	-48.628	
		3.000			-2582.242			.000

9	1	.000			-1312.032			.000
		.000	32.212	-134.433		26.862	42.438	
		3.000	32.212	-37.797		26.862	123.024	
		3.000			-1312.032			.000

10	1	.000			-2669.294			.000
		.000	59.408	-159.933		-14.129	80.871	
		3.000	59.408	18.293		-14.129	38.484	
		3.000			-2669.294			.000

11	1	.000			-2658.031			.000
		.000	59.419	-159.942		-13.678	80.448	
		3.000	59.419	18.315		-13.678	39.415	
		3.000			-2658.031			.000

12	1	.000			-1580.356			.000
		.000	32.222	-134.443		-29.534	95.315	
		3.000	32.222	-37.776		-29.534	6.713	
		3.000			-1580.356			.000

13	1	.000							
		.000	50.667	-138.940		-17.070	65.855		.000
		3.000	50.667	13.061		-17.070	14.645		
		3.000						-711.067	.000
14	1	.000							
		.000	4.971	-66.843		-101.003	190.656		.000
		3.000	4.971	-51.929		-101.003	-112.351		
		3.000						-1781.407	.000
15	1	.000							
		.000	4.942	-66.803		-102.730	191.734		.000
		3.000	4.942	-51.978		-102.730	-116.456		
		3.000						-1772.199	.000
16	1	.000							
		.000	50.638	-138.901		-118.816	224.650		.000
		3.000	50.638	13.012		-118.816	-131.798		
		3.000						-934.312	.000
17	1	.000							
		.000	154.591	-290.183		29.036	-6.722		.000
		3.000	154.591	173.589		29.036	80.386		
		3.000						-1834.762	.000
18	1	.000							
		.000	197.894	-416.289		-133.319	279.588		.000
		3.000	197.894	177.393		-133.319	-120.368		
		3.000						-3952.077	.000
19	1	.000							
		.000	197.894	-416.289		-131.528	274.466		.000
		3.000	197.894	177.393		-131.528	-120.118		
		3.000						-3953.734	.000
20	1	.000							
		.000	154.591	-290.183		-166.995	300.023		.000
		3.000	154.591	173.589		-166.995	-200.962		
		3.000						-2068.252	.000
21	1	.000							
		.000	152.468	-297.735		-17.025	65.795		.000
		3.000	152.468	159.669		-17.025	14.720		
		3.000						-1056.164	.000
22	1	.000							
		.000	201.401	-374.205		-100.959	190.598		.000
		3.000	201.401	229.998		-100.959	-112.279		
		3.000						-2143.054	.000
23	1	.000							
		.000	201.430	-374.244		-102.773	191.792		.000
		3.000	201.430	230.047		-102.773	-116.529		
		3.000						-2133.846	.000
24	1	.000							
		.000	152.497	-297.774		-118.861	224.711		.000
		3.000	152.497	159.718		-118.861	-131.873		
		3.000						-1279.409	.000
25	1	.000							
		.000	34.243	-43.266		-5.175	2.073		.000
		3.000	34.243	59.462		-5.175	-13.452		
		3.000						-556.490	.000
26	1	.000							
								-1348.593	.000

		.000	-7.810	18.308	-92.414	132.170	
		3.000	-7.810	-5.122	-92.414	-145.072	
		3.000		-1348.593			.000
27	-----						
	1	.000		-1342.814			.000
		.000	-7.856	18.372	-97.101	139.050	.000
		3.000	-7.856	-5.195	-97.101	-152.254	
		3.000		-1342.814			.000
28	-----						
	1	.000		-705.646			.000
		.000	34.197	-43.203	-103.339	145.158	.000
		3.000	34.197	59.390	-103.339	-164.860	
		3.000		-705.646			.000
29	-----						
	1	.000		-1384.504			.000
		.000	146.928	-212.009	37.495	-60.446	.000
		3.000	146.928	228.775	37.495	52.038	
		3.000		-1384.504			.000
30	-----						
	1	.000		-2964.590			.000
		.000	168.613	-235.087	-109.393	150.109	.000
		3.000	168.613	270.752	-109.393	-178.070	
		3.000		-2964.590			.000
31	-----						
	1	.000		-2966.231			.000
		.000	168.613	-235.087	-110.929	152.902	.000
		3.000	168.613	270.752	-110.929	-179.883	
		3.000		-2966.231			.000
32	-----						
	1	.000		-1540.652			.000
		.000	146.928	-212.009	-148.551	211.540	.000
		3.000	146.928	228.775	-148.551	-234.114	
		3.000		-1540.652			.000
33	-----						
	1	.000		-788.821			.000
		.000	132.561	-186.609	-5.105	1.974	.000
		3.000	132.561	211.074	-5.105	-13.340	
		3.000		-788.821			.000
34	-----						
	1	.000		-1592.336			.000
		.000	178.615	-254.238	-92.347	132.075	.000
		3.000	178.615	281.606	-92.347	-144.966	
		3.000		-1592.336			.000
35	-----						
	1	.000		-1586.557			.000
		.000	178.661	-254.302	-97.168	139.144	.000
		3.000	178.661	281.679	-97.168	-152.361	
		3.000		-1586.557			.000
36	-----						
	1	.000		-937.978			.000
		.000	132.606	-186.673	-103.409	145.256	.000
		3.000	132.606	211.146	-103.409	-164.972	
		3.000		-937.978			.000
37	-----						
	1	.000		-394.280			.000
		.000	18.222	17.977	3.711	-37.841	.000
		3.000	18.222	72.642	3.711	-26.709	
		3.000		-394.280			.000
38	-----						
	1	.000		-912.654			.000
		.000	-20.077	79.041	-68.096	79.310	.000
		3.000	-20.077	18.809	-68.096	-124.978	
		3.000		-912.654			.000
39	-----						
	1	.000		-909.591			.000
		.000	-20.124	79.115	-75.322	88.873	.000
		3.000	-20.124	18.742	-75.322	-137.094	

	1	.000							
		.000	76.346	-69.176		121.408	-168.329		.000
		3.000	76.346	159.862		121.408	195.897		
		3.000						-471.490	.000
54	-----								
	1	.000							
		.000	68.669	-25.189		-43.056	9.720		.000
		3.000	68.669	180.818		-43.056	-119.448		
		3.000						-999.719	.000
55	-----								
	1	.000							
		.000	68.669	-25.189		-42.259	15.596		.000
		3.000	68.669	180.818		-42.259	-111.179		
		3.000						-1000.319	.000
56	-----								
	1	.000							
		.000	76.346	-69.176		-160.997	173.078		.000
		3.000	76.346	159.862		-160.997	-309.913		
		3.000						-503.842	.000
57	-----								
	1	.000							
		.000	106.902	-97.082		57.556	-90.498		.000
		3.000	106.902	223.625		57.556	82.170		
		3.000						-254.092	.000
58	-----								
	1	.000							
		.000	174.400	-178.742		-43.080	34.801		.000
		3.000	174.400	344.456		-43.080	-94.439		
		3.000						-513.944	.000
59	-----								
	1	.000							
		.000	174.488	-178.848		-50.562	47.232		.000
		3.000	174.488	344.615		-50.562	-104.456		
		3.000						-512.007	.000
60	-----								
	1	.000							
		.000	106.991	-97.187		-94.670	92.558		.000
		3.000	106.991	223.785		-94.670	-191.452		
		3.000						-284.223	.000

PROGRAM:SAP90/FILE:\11d.F3F

5 storey building-Bay size 5*5 //Fc=45MPa

FRAME ELEMENT FORCES

ELT ID	LOAD COMB	DIST ENDI	1-2 PLANE		AXIAL FORCE	1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT		SHEAR	MOMENT	
1	1	.000			-328.624			.000
		.000	-4.506	-6.700		5.721	1.652	
		3.000	-4.506	-20.218		5.721	18.815	
		3.000			-328.624			.000
2	1	.000			-914.952			.000
		.000	-8.779	-2.493		-6.078	13.267	
		3.000	-8.779	-28.831		-6.078	-4.968	
		3.000			-914.952			.000
3	1	.000			-891.142			.000
		.000	-8.822	-2.451		-5.151	12.354	
		3.000	-8.822	-28.917		-5.151	-3.098	
		3.000			-891.142			.000
4	1	.000			-492.175			.000
		.000	-4.551	-6.655		-8.505	15.656	
		3.000	-4.551	-20.309		-8.505	-9.860	
		3.000			-492.175			.000
5	1	.000			-960.496			.000
		.000	9.459	-20.446		10.373	-2.928	
		3.000	9.459	7.929		10.373	28.192	
		3.000			-960.496			.000
6	1	.000			-2263.793			.000
		.000	1.021	-36.187		-2.440	25.392	
		3.000	1.021	-33.122		-2.440	18.073	
		3.000			-2263.793			.000
7	1	.000			-2284.909			.000
		.000	1.021	-36.187		1.955	21.120	
		3.000	1.021	-33.122		1.955	26.983	
		3.000			-2284.909			.000
8	1	.000			-1159.433			.000
		.000	9.459	-20.446		-14.627	21.682	
		3.000	9.459	7.929		-14.627	-22.198	
		3.000			-1159.433			.000
9	1	.000			-571.635			.000
		.000	9.588	-20.574		5.786	1.588	
		3.000	9.588	8.189		5.786	18.947	
		3.000			-571.635			.000
10	1	.000			-1216.847			.000
		.000	16.271	-27.153		-6.023	13.212	
		3.000	16.271	21.661		-6.023	-4.856	
		3.000			-1216.847			.000
11	1	.000			-1193.037			.000
		.000	16.314	-27.195		-5.207	12.409	
		3.000	16.314	21.747		-5.207	-3.211	
		3.000			-1193.037			.000
12	1	.000			-735.185			.000
		.000	9.633	-20.618		-8.571	15.720	
		3.000	9.633	8.280		-8.571	-9.991	
		3.000			-735.185			.000

		.000	14.840	-20.606	-35.105	51.805	
		3.000	14.840	23.913	-35.105	-53.509	
		3.000					.000
27				-558.513			
	1	.000		-548.281			.000
		.000	14.663	-20.352	-36.225	53.350	
		3.000	14.663	23.638	-36.225	-55.324	
		3.000		-548.281			.000
28							
	1	.000		-301.191			.000
		.000	19.402	-27.165	-35.734	51.634	
		3.000	19.402	31.041	-35.734	-55.568	
		3.000		-301.191			.000
29							
	1	.000		-581.840			.000
		.000	55.110	-81.553	-2.388	2.527	
		3.000	55.110	83.776	-2.388	-4.637	
		3.000		-581.840			.000
30							
	1	.000		-1359.315			.000
		.000	81.014	-113.131	-58.393	81.549	
		3.000	81.014	129.910	-58.393	-93.630	
		3.000		-1359.315			.000
31							
	1	.000		-1371.000			.000
		.000	81.014	-113.131	-53.841	75.133	
		3.000	81.014	129.910	-53.841	-86.389	
		3.000		-1371.000			.000
32							
	1	.000		-682.986			.000
		.000	55.110	-81.553	-46.902	68.543	
		3.000	55.110	83.776	-46.902	-72.163	
		3.000		-682.986			.000
33							
	1	.000		-343.148			.000
		.000	46.878	-67.653	-8.497	11.520	
		3.000	46.878	72.982	-8.497	-13.971	
		3.000		-343.148			.000
34							
	1	.000		-713.479			.000
		.000	59.457	-86.834	-34.875	51.475	
		3.000	59.457	91.538	-34.875	-53.151	
		3.000		-713.479			.000
35							
	1	.000		-703.247			.000
		.000	59.634	-87.088	-36.454	53.681	
		3.000	59.634	91.814	-36.454	-55.683	
		3.000		-703.247			.000
36							
	1	.000		-426.742			.000
		.000	47.064	-67.920	-36.003	52.021	
		3.000	47.064	73.273	-36.003	-55.989	
		3.000		-426.742			.000
37							
	1	.000		-162.426			.000
		.000	11.442	-12.328	-3.364	1.587	
		3.000	11.442	22.000	-3.364	-8.505	
		3.000		-162.426			.000
38							
	1	.000		-383.541			.000
		.000	6.022	-4.248	-25.552	36.618	
		3.000	6.022	13.820	-25.552	-40.038	
		3.000		-383.541			.000
39							
	1	.000		-379.472			.000
		.000	5.828	-3.954	-27.978	39.719	
		3.000	5.828	13.529	-27.978	-44.216	

	1	.000							
		.000	24.272	-32.693		19.368		-27.429	.000
		3.000	24.272	40.124		19.368		30.675	
		3.000						-204.537	.000
54	-----								
	1	.000							
		.000	31.892	-30.982		-27.198		26.613	.000
		3.000	31.892	64.693		-27.198		-54.981	
		3.000						-464.556	.000
55	-----								
	1	.000							
		.000	31.892	-30.982		-17.851		17.955	.000
		3.000	31.892	64.693		-17.851		-35.599	
		3.000						-467.405	.000
56	-----								
	1	.000							
		.000	24.272	-32.693		-39.752		51.450	.000
		3.000	24.272	40.124		-39.752		-67.805	
		3.000						-219.646	.000
57	-----								
	1	.000							
		.000	32.968	-41.199		10.383		-15.452	.000
		3.000	32.968	57.706		10.383		15.696	
		3.000						-106.388	.000
58	-----								
	1	.000							
		.000	45.274	-58.185		-14.592		18.839	.000
		3.000	45.274	77.638		-14.592		-24.937	
		3.000						-220.933	.000
59	-----								
	1	.000							
		.000	45.548	-58.554		-16.717		23.001	.000
		3.000	45.548	78.091		-16.717		-27.150	
		3.000						-219.933	.000
60	-----								
	1	.000							
		.000	33.266	-41.596		-28.271		35.687	.000
		3.000	33.266	58.202		-28.271		-49.125	
		3.000						-118.608	.000

PROGRAM:SAP90/FILE:\33d.F3F

5 storey building-Bay size 7*7 //Fc=45MPa

FRAME ELEMENT FORCES

ELT ID	LOAD COMB	DIST ENDI	1-2 PLANE		AXIAL FORCE	1-3 PLANE		AXIAL TORQ
			SHEAR	MOMENT		SHEAR	MOMENT	
1								
	1	.000			-895.250			.000
		.000	-23.497	-64.242		26.256	31.136	
		3.000	-23.497	-134.733		26.256	109.903	
		3.000			-895.250			.000
2								
	1	.000			-2234.524			.000
		.000	-48.773	-40.061		-14.430	70.058	
		3.000	-48.773	-186.380		-14.430	26.770	
		3.000			-2234.524			.000
3								
	1	.000			-2220.649			.000
		.000	-48.784	-40.051		-13.251	68.931	
		3.000	-48.784	-186.403		-13.251	29.177	
		3.000			-2220.649			.000
4								
	1	.000			-1158.440			.000
		.000	-23.508	-64.231		-29.437	84.415	
		3.000	-23.508	-134.756		-29.437	-3.895	
		3.000			-1158.440			.000
5								
	1	.000			-2303.729			.000
		.000	23.601	-109.300		51.607	6.883	
		3.000	23.601	-38.495		51.607	161.705	
		3.000			-2303.729			.000
6								
	1	.000			-4974.873			.000
		.000	10.541	-189.708		-6.779	123.004	
		3.000	10.541	-158.085		-6.779	102.668	
		3.000			-4974.873			.000
7								
	1	.000			-4977.873			.000
		.000	10.541	-189.708		-2.389	118.888	
		3.000	10.541	-158.085		-2.389	111.722	
		3.000			-4977.873			.000
8								
	1	.000			-2582.575			.000
		.000	23.601	-109.300		-55.762	109.599	
		3.000	23.601	-38.495		-55.762	-57.686	
		3.000			-2582.575			.000
9								
	1	.000			-1299.767			.000
		.000	32.117	-117.447		26.273	31.120	
		3.000	32.117	-21.094		26.273	109.938	
		3.000			-1299.767			.000
10								
	1	.000			-2664.295			.000
		.000	58.935	-143.102		-14.413	70.043	
		3.000	58.935	33.703		-14.413	26.803	
		3.000			-2664.295			.000
11								
	1	.000			-2650.420			.000
		.000	58.946	-143.113		-13.268	68.947	
		3.000	58.946	33.726		-13.268	29.144	
		3.000			-2650.420			.000
12								
	1	.000			-1562.958			.000
		.000	32.129	-117.457		-29.454	84.432	
		3.000	32.129	-21.071		-29.454	-3.930	
		3.000			-1562.958			.000

		.000	-8.082	18.849	-90.104	128.997	
		3.000	-8.082	-5.398	-90.104	-141.315	
		3.000		-1348.702			.000
27	1	.000		-1341.476			.000
		.000	-8.130	18.916	-92.884	133.250	
		3.000	-8.130	-5.475	-92.884	-145.403	
		3.000		-1341.476			.000
28	1	.000		-700.203			.000
		.000	32.015	-39.899	-100.096	140.694	
		3.000	32.015	56.146	-100.096	-159.595	
		3.000		-700.203			.000
29	1	.000		-1386.753			.000
		.000	142.097	-205.056	36.674	-59.153	
		3.000	142.097	221.234	36.674	50.870	
		3.000		-1386.753			.000
30	1	.000		-2984.000			.000
		.000	166.978	-230.159	-110.411	150.000	
		3.000	166.978	270.775	-110.411	-181.233	
		3.000		-2984.000			.000
31	1	.000		-2986.288			.000
		.000	166.978	-230.159	-108.421	147.999	
		3.000	166.978	270.775	-108.421	-177.264	
		3.000		-2986.288			.000
32	1	.000		-1540.159			.000
		.000	142.097	-205.056	-144.263	205.690	
		3.000	142.097	221.234	-144.263	-227.098	
		3.000		-1540.159			.000
33	1	.000		-779.919			.000
		.000	128.064	-180.068	-4.155	.743	
		3.000	128.064	204.124	-4.155	-11.723	
		3.000		-779.919			.000
34	1	.000		-1587.407			.000
		.000	173.391	-246.786	-90.034	128.897	
		3.000	173.391	273.387	-90.034	-141.204	
		3.000		-1587.407			.000
35	1	.000		-1580.182			.000
		.000	173.439	-246.854	-92.954	133.349	
		3.000	173.439	273.463	-92.954	-145.514	
		3.000		-1580.182			.000
36	1	.000		-924.517			.000
		.000	128.112	-180.136	-100.170	140.798	
		3.000	128.112	204.201	-100.170	-159.712	
		3.000		-924.517			.000
37	1	.000		-393.990			.000
		.000	16.931	13.858	4.043	-34.076	
		3.000	16.931	64.651	4.043	-21.948	
		3.000		-393.990			.000
38	1	.000		-913.085			.000
		.000	-19.896	72.305	-66.396	80.631	
		3.000	-19.896	12.615	-66.396	-118.556	
		3.000		-913.085			.000
39	1	.000		-909.222			.000
		.000	-19.946	72.382	-71.829	87.257	
		3.000	-19.946	12.545	-71.829	-128.230	

		3.000			-909.222			.000
40	-----							
	1	.000			-470.920			.000
		.000	16.882	13.935		-82.275	104.543	
		3.000	16.882	64.581		-82.275	-142.282	
		3.000			-470.920			.000
41	-----							
	1	.000			-934.293			.000
		.000	107.515	-132.546		41.327	-93.324	
		3.000	107.515	189.998		41.327	30.657	
		3.000			-934.293			.000
42	-----							
	1	.000			-1992.530			.000
		.000	124.303	-115.462		-79.871	73.796	
		3.000	124.303	257.447		-79.871	-165.818	
		3.000			-1992.530			.000
43	-----							
	1	.000			-1994.144			.000
		.000	124.303	-115.462		-82.770	74.410	
		3.000	124.303	257.447		-82.770	-173.899	
		3.000			-1994.144			.000
44	-----							
	1	.000			-1016.256			.000
		.000	107.515	-132.546		-121.583	167.398	
		3.000	107.515	189.998		-121.583	-197.350	
		3.000			-1016.256			.000
45	-----							
	1	.000			-514.440			.000
		.000	103.551	-125.068		4.120	-34.197	
		3.000	103.551	185.583		4.120	-21.836	
		3.000			-514.440			.000
46	-----							
	1	.000			-1041.879			.000
		.000	143.480	-189.168		-66.320	80.515	
		3.000	143.480	241.272		-66.320	-118.444	
		3.000			-1041.879			.000
47	-----							
	1	.000			-1038.016			.000
		.000	143.529	-189.246		-71.905	87.373	
		3.000	143.529	241.343		-71.905	-128.341	
		3.000			-1038.016			.000
48	-----							
	1	.000			-591.369			.000
		.000	103.600	-125.146		-82.353	104.664	
		3.000	103.600	185.653		-82.353	-142.394	
		3.000			-591.369			.000
49	-----							
	1	.000			-206.066			.000
		.000	-43.051	78.770		54.756	-84.231	
		3.000	-43.051	-50.382		54.756	80.035	
		3.000			-206.066			.000
50	-----							
	1	.000			-462.529			.000
		.000	-103.166	153.084		-42.933	38.445	
		3.000	-103.166	-156.414		-42.933	-90.353	
		3.000			-462.529			.000
51	-----							
	1	.000			-460.293			.000
		.000	-103.257	153.194		-46.248	46.093	
		3.000	-103.257	-156.577		-46.248	-92.651	
		3.000			-460.293			.000
52	-----							
	1	.000			-234.154			.000
		.000	-43.143	78.881		-92.104	94.021	
		3.000	-43.143	-50.548		-92.104	-182.291	
		3.000			-234.154			.000
53	-----							

1	.000			-472.833			.000
	.000	72.458	-70.422		115.368	-159.153	
	3.000	72.458	146.952		115.368	186.949	
	3.000			-472.833			.000
54	-----						
1	.000			-1007.841			.000
	.000	65.291	-23.708		-45.167	13.707	
	3.000	65.291	172.164		-45.167	-121.795	
	3.000			-1007.841			.000
55	-----						
1	.000			-1008.664			.000
	.000	65.291	-23.708		-36.964	11.378	
	3.000	65.291	172.164		-36.964	-99.514	
	3.000			-1008.664			.000
56	-----						
1	.000			-503.536			.000
	.000	72.458	-70.422		-155.607	172.236	
	3.000	72.458	146.952		-155.607	-294.585	
	3.000			-503.536			.000
57	-----						
1	.000			-250.341			.000
	.000	103.974	-99.950		54.894	-84.402	
	3.000	103.974	211.973		54.894	80.281	
	3.000			-250.341			.000
58	-----						
1	.000			-511.309			.000
	.000	168.873	-179.527		-42.805	38.284	
	3.000	168.873	327.093		-42.805	-90.130	
	3.000			-511.309			.000
59	-----						
1	.000			-509.074			.000
	.000	168.964	-179.637		-46.376	46.254	
	3.000	168.964	327.256		-46.376	-92.874	
	3.000			-509.074			.000
60	-----						
1	.000			-278.429			.000
	.000	104.066	-100.061		-92.243	94.191	
	3.000	104.066	212.138		-92.243	-182.537	
	3.000			-278.429			.000

Appendix D

```

CLS
K8 = .45: REM ***** Concrete stress factor
k9 = .9: REM *****Neutral axis depth factor
PRINT "AXIAL LOAD N, KN"
INPUT N
PRINT "MOMENT MX, KN.m"
INPUT MX
PRINT "MOMENT MY, KN.m"
INPUT MY
FU = 45: FY = 460: D1 = 60: D2 = 60: H = 600: B = 600
B4 = B1: H4 = H: D4 = D1: D5 = D2
E1 = FY / 1.15 / 200000:

GOSUB 5000: REM *** REM Calculation Of increased reinforcement
N = N * 10 ^ 3: M = M * 10 ^ 6: EM = H / 20
IF EM > 20 THEN EM = 20: REM *** Minimum moment
IF M < N * EM THEN M = N * EM
D = H - D1

REM*****Check of unreinforced section

IF N = 0 THEN 360
REM *****Calculate depth of stress block with no steel

S = N / B / K8 / FU
IF S > H THEN 360
REM ** Check if M < moment of resistance of unreinforced section

IF N * (H - S) / 2 > M THEN 940

360 REM * Iteration with X decreasing to balance the moment

X = 2.333 * D: U = 10
380 X = X + H / U
390 X = X - H / U
IF X < .9 * D THEN 570
IF X > H / k9 THEN S = H
IF X < H / k9 THEN S = k9 * X
IF X = D THEN F3 = 0: GOTO 480
E = ABS(.0035 * (D - X) / X)
IF X > D THEN GOSUB 4000: REM *** Steel compressive stress
subroutine
IF X < D THEN GOSUB 4030: REM *** Steel tensile stress
subroutine

F3 = F7
480 E = .0035 * (X - D1) / X
GOSUB 4000: REM *** Steel compressive stress subroutine

F4 = F7
ST = 2 * (N - K8 * FU * B * S) / (F3 + F4)
IF ST <= 0 THEN 390
M3 = K8 * FU * B * S * (H - S) / 2 + ST * (F4 - F3) * (H / 2 -
D1) / 2
IF M3 < M THEN 390
IF M * 1.02 > M3 THEN 790
X = X + H / U: U = 10 * U: GOTO 380

570 REM * Iteration with X increasing to balance the axial load

```

```

X = D1 + 1: U = 10
590 X = X - H / U
600 X = X + H / U
IF X > H / k9 THEN S = H
IF X < H / k9 THEN S = k9 * X
IF X = D THEN F3 = 0: GOTO 680
E = ABS(.0035 * (D - X) / X)
IF X > D THEN GOSUB 4000: REM *** Steel compressive stress
subroutine
IF X < D THEN GOSUB 4030: REM *** Steel tensile stress
subroutine

F3 = F7
680 E = .0035 * (X - D1) / X
GOSUB 4000: REM *** Steel compressive stress subroutine

F4 = F7
ST = 2 * (M - K8 * FU * B * S * (H - S) / 2) / (F4 - F3) / (H / 2
- D1)
IF ST < 0 THEN 600
N2 = K8 * FU * B * S + ST * (F3 + F4) / 2
IF N2 < N THEN 600
IF X = D1 + 1 AND N2 > N THEN 790
IF N * 1.02 > N2 THEN 790
IF N = 0 AND N2 - N < .1 THEN 790
X = X - H / U: U = 10 * U: GOTO 590
790 N2 = K8 * FU * B * S + ST * (F3 + F4) / 2
M3 = K8 * FU * B * S * (H - S) / 2 + ST * (F4 - F3) * (H / 2 -
D1) / 2

REM** print output

PRINT "TOTAL STEEL AREA sq.mm"; INT(ST * 100) / 100
IF 100 * ST / B / H < .4 THEN 940
PRINT " AXIAL STRENGTH (KN)"; INT(N2 / 10) / 100
PRINT " MOMENT STRENGTH (KN.m)"; INT(M3 / 10 ^ 4) / 100
END
940 PRINT "MINIMUM STEEL AREA REQUIRED= 0.4% OF B*H="; .4 * B * H /
100; "sq.mm"
END

REM * Subroutine for steel compressive stress

4000 IF E >= E1 THEN F7 = FY / 1.15: GOTO 4060
IF E <= E1 THEN F7 = 200000 * E: GOTO 4060

REM * Subroutine for steel tensile stress

4030 IF E >= E1 THEN F7 = -(FY / 1.15): GOTO 4060
IF E <= E1 THEN F7 = -(200000 * E)
4060 RETURN

REM ** Subroutine to calculate incresed moment coefficient

5000 C1 = N / B / H / FU * 1000
IF C1 > .6 THEN C2 = .3: GOTO 5060
C2 = .3 + .7 / .6 * (.6 - C1)
5060 H1 = H - D1: B1 = B - D2
IF MX / H1 < MY / B1 THEN 5100
M = MX + C2 * (H1 / B1) * MY: AX$ = "X": REM X axis bending

```

```
GOTO 5120  
5100 M = MY + C2 * (B1 / H1) * MX: AX$ = "Y": REM Y axis bending  
  
H = B1 + D2: B = H1 + D1: D1 = D2  
5120 RETURN
```

Bill of quantities for columns of a 15 story structure
 $f_c=25$ Mpa $f_y=460$ Mpa span length= 5m

Floor level	Column cross section (sq. mm)			Reinforcement cross section (sq. mm)		
	interior	exterior	TOTAL	interior	exterior	TOTAL
1	640000	360000	4880000	2560	1440	19520
2	640000	360000	4880000	2560	1440	19520
3	640000	360000	4880000	2560	1440	19520
4	640000	360000	4880000	2560	1440	19520
5	640000	360000	4880000	2560	1440	19520
6	490000	250000	3480000	2109	2794	32158
7	490000	250000	3480000	1960	2417	28090
8	490000	250000	3480000	1960	1909	23010
9	490000	250000	3480000	1960	1372	17640
10	490000	250000	3480000	1960	1466	18580
11	360000	160000	2320000	1440	2046	23340
12	360000	160000	2320000	1440	1050	13380
13	360000	160000	2320000	1440	1560	18480
14	160000	90000	1220000	809	1486	16478
15	160000	90000	1220000	640	1552	16800
Total concrete area			51200000	Total steel area		305556

Bill of quantities for columns of a 10 story structure
 $f_c=25$ Mpa $f_y=460$ Mpa span length= 5m

Floor level	Column cross section (sq. mm)			Reinforcement cross section (sq. mm)		
	interior	exterior	TOTAL	interior	exterior	TOTAL
1	490000	250000	3480000	1960	1000	13920
2	490000	250000	3480000	1960	1000	13920
3	490000	250000	3480000	1960	1000	13920
4	490000	250000	3480000	1960	1000	13920
5	490000	250000	3480000	1960	1287	16790
6	250000	160000	2100000	2226	2310	27552
7	250000	160000	2100000	1236	1976	22232
8	250000	160000	2100000	1000	1658	18580
9	250000	160000	2100000	1000	1346	15460
10	250000	160000	2100000	1000	1665	18650
Total concrete area			27900000	Total steel area		174944

Bill of quantities for columns of a 20 story structure
 $f_c=25$ Mpa $f_y=460$ Mpa span length= 5m

Floor level	Column cross section (sq. mm)			Reinforcement cross section (sq. mm)		
	interior	exterior	TOTAL	interior	exterior	TOTAL
1	810000	490000	6520000	3240	1960	26080
2	810000	490000	6520000	3240	1960	26080
3	810000	490000	6520000	3240	1960	26080
4	810000	490000	6520000	3240	1960	26080
5	810000	490000	6520000	3240	2027	26750
6	640000	360000	4880000	3522	3457	41614
7	640000	360000	4880000	2560	3095	36070
8	640000	360000	4880000	2560	1440	19520
9	640000	360000	4880000	2560	1440	19520
10	640000	360000	4880000	2560	1458	19700
11	490000	250000	3480000	1960	3100	34920
12	490000	250000	3480000	1960	2488	28800
13	490000	250000	3480000	1960	2258	26500
14	250000	160000	2100000	3936	3701	44882
15	250000	160000	2100000	2709	2952	34938
16	250000	160000	2100000	1529	2652	29578
17	160000	90000	1220000	3456	3203	38942
18	160000	90000	1220000	2157	2483	29144
19	160000	90000	1220000	640	1836	19640
20	160000	90000	1220000	640	1765	18930
Total concrete area			78620000	Total steel area		573768

Bill of quantities for columns of a 5 story structure
 $f_c=25$ Mpa $f_y=460$ Mpa span length= 5m

Floor level	Column cross section (sq. mm)			Reinforcement cross section (sq. mm)		
	interior	exterior	TOTAL	interior	exterior	TOTAL
1	250000	160000	2100000	1000	640	8400
2	250000	160000	2100000	1404	1586	18668
3	250000	160000	2100000	1000	1326	15260
4	250000	160000	2100000	1000	1232	14320
5	250000	160000	2100000	1000	1633	18330
Total concrete area			10500000	Total steel area		74978

Bill of quantities for columns of a 20 story structure
 $f_c=25$ Mpa $f_y=460$ Mpa span length= 7m

Floor level	Column cross section (sq. mm)			Reinforcement cross section (sq. mm)		
	interior	exterior	TOTAL	interior	exterior	TOTAL
1	1210000	810000	10520000	15531	4990	80962
2	1210000	810000	10520000	13850	4541	73110
3	1210000	810000	10520000	13141	4402	70302
4	1210000	810000	10520000	13756	5587	83382
5	1210000	810000	10520000	16563	8687	119996
6	1000000	640000	8400000	16457	9761	130524
7	1000000	640000	8400000	11264	7667	99198
8	1000000	640000	8400000	7438	5618	71056
9	1000000	640000	8400000	5138	4572	55996
10	1000000	640000	8400000	4000	4030	48300
11	810000	490000	6520000	6430	6202	74880
12	810000	490000	6520000	4200	5021	58610
13	810000	490000	6520000	3240	4168	48160
14	490000	360000	4580000	7854	5778	73488
15	490000	360000	4580000	5274	4352	54068
16	490000	360000	4580000	2425	3619	41040
17	360000	250000	3220000	3833	4156	49226
18	360000	250000	3220000	1440	3005	32930
19	360000	250000	3220000	1440	2250	25380
20	360000	250000	3220000	1440	3913	42010
Total concrete area			140780000	Total steel area		1332618

Bill of quantities for columns of a 5 story structure
 $f_c=25$ Mpa $f_y=460$ Mpa span length= 7m

Floor level	Column cross section (sq. mm)			Reinforcement cross section (sq. mm)		
	interior	exterior	TOTAL	interior	exterior	TOTAL
1	490000	360000	4580000	1960	1440	18320
2	490000	360000	4580000	1960	2175	25670
3	490000	360000	4580000	1960	1440	18320
4	490000	360000	4580000	1960	1569	19610
5	490000	360000	4580000	1960	1440	18320
Total concrete area			22900000	Total steel area		100240

Bill of quantities for columns of a 15 story structure
 $f_c=25$ Mpa $f_y=460$ Mpa span length= 7m

Floor level	Column cross section (sq. mm)			Reinforcement cross section (sq. mm)		
	interior	exterior	TOTAL	interior	exterior	TOTAL
1	1000000	640000	8400000	9803	2560	45206
2	1000000	640000	8400000	7874	2560	41348
3	1000000	640000	8400000	6840	2560	39280
4	1000000	640000	8400000	7431	2584	40702
5	1000000	640000	8400000	10184	5877	79138
6	810000	490000	6520000	9544	6422	83308
7	810000	490000	6520000	4526	4757	56622
8	810000	490000	6520000	3240	3735	43830
9	810000	490000	6520000	3240	2832	34800
10	810000	490000	6520000	3240	2456	31040
11	640000	360000	4880000	2560	3441	39530
12	640000	360000	4880000	2560	2408	29200
13	640000	360000	4880000	2560	2509	30210
14	360000	250000	3220000	1400	2248	25280
15	360000	250000	3220000	1440	3787	40750
Total concrete area			95680000	Total steel area		660244

Bill of quantities for columns of a 10 story structure
 $f_c=25$ Mpa $f_y=460$ Mpa span length= 7m

Floor level	Column cross section (sq. mm)			Reinforcement cross section (sq. mm)		
	interior	exterior	TOTAL	interior	exterior	TOTAL
1	810000	490000	6520000	3240	1960	26080
2	810000	490000	6520000	3240	1960	26080
3	810000	490000	6520000	3240	1960	26080
4	810000	490000	6520000	3240	1960	26080
5	810000	490000	6520000	3240	2571	32190
6	490000	360000	4580000	3703	3000	37406
7	490000	360000	4580000	1960	2579	29710
8	490000	360000	4580000	1960	2461	28530
9	490000	360000	4580000	1960	2037	24290
10	490000	360000	4580000	1960	3695	40870
Total concrete area			55500000	Total steel area		297316

Bill of quantities for columns of a 20 story structure
 $f_c=45$ Mpa $f_y=460$ Mpa span length= 5m

Floor level	Column cross section (sq. mm)			Reinforcement cross section (sq. mm)		
	interior	exterior	TOTAL	interior	exterior	TOTAL
1	490000	360000	4580000	1960	1440	18320
2	490000	360000	4580000	1960	1440	18320
3	490000	360000	4580000	1960	1440	18320
4	490000	360000	4580000	1960	1440	18320
5	490000	360000	4580000	1960	1440	18320
6	360000	250000	3220000	1977	1000	13954
7	360000	250000	3220000	1440	1765	20530
8	360000	250000	3220000	1440	1237	15250
9	360000	250000	3220000	1440	1000	12880
10	360000	250000	3220000	1440	1079	13670
11	250000	160000	2100000	2047	3412	38214
12	250000	160000	2100000	1881	2900	32762
13	250000	160000	2100000	1000	2883	30830
14	160000	90000	1220000	4576	4642	55572
15	160000	90000	1220000	2957	4078	46694
16	160000	90000	1220000	1999	3150	35498
17	122500	90000	1145000	2203	2645	30856
18	122500	90000	1145000	490	2334	24320
19	122500	90000	1145000	490	1764	18620
20	122500	90000	1145000	490	2162	22600
Total concrete area			53540000	Total steel area		503850

Bill of quantities for columns of a 5 story structure
 $f_c=45$ Mpa $f_y=460$ Mpa span length= 5m

Floor level	Column cross section (sq. mm)			Reinforcement cross section (sq. mm)		
	interior	exterior	TOTAL	interior	exterior	TOTAL
1	160000	90000	1220000	640	360	4880
2	160000	90000	1220000	2155	2529	29600
3	160000	90000	1220000	803	2220	23806
4	160000	90000	1220000	692	1967	21054
5	160000	90000	1220000	647	2118	22474
Total concrete area			6100000	Total steel area		101814

Bill of quantities for columns of a 15 story structure
 $f_c=45$ Mpa $f_y=460$ Mpa span length= 5m

Floor level	Column cross section (sq. mm)			Reinforcement cross section (sq. mm)		
	interior	exterior	TOTAL	interior	exterior	TOTAL
1	360000	250000	3220000	1440	1000	12880
2	360000	250000	3220000	1440	1000	12880
3	360000	250000	3220000	1440	1000	12880
4	360000	250000	3220000	1440	1000	12880
5	360000	250000	3220000	1440	1400	16880
6	250000	160000	2100000	3372	3274	39484
7	250000	160000	2100000	1582	2969	32854
8	250000	160000	2100000	1000	2511	27110
9	250000	160000	2100000	1000	1844	20440
10	250000	160000	2100000	1000	2222	24220
11	160000	90000	1220000	2376	3466	39412
12	160000	90000	1220000	1289	2776	30338
13	160000	90000	1220000	799	2267	24268
14	122500	90000	1145000	490	2024	21220
15	122500	90000	1145000	490	2249	23470
Total concrete area			32550000	Total steel area		351216

Bill of quantities for columns of a 10 story structure
 $f_c=45$ Mpa $f_y=460$ Mpa span length= 5m

Floor level	Column cross section (sq. mm)			Reinforcement cross section (sq. mm)		
	interior	exterior	TOTAL	interior	exterior	TOTAL
1	250000	160000	2100000	1000	640	8400
2	250000	160000	2100000	1000	640	8400
3	250000	160000	2100000	1000	640	8400
4	250000	160000	2100000	1000	640	8400
5	250000	160000	2100000	1000	2114	23140
6	160000	90000	1220000	3319	3731	43948
7	160000	90000	1220000	1808	3199	35606
8	160000	90000	1220000	1071	2647	28612
9	160000	90000	1220000	770	2102	22560
10	160000	90000	1220000	770	2249	24030
Total concrete area			16600000	Total steel area		211496

Bill of quantities for columns of a 20 story structure
 $f_c=45$ Mpa $f_y=460$ Mpa span length= 7m

Floor level	Column cross section (sq. mm)			Reinforcement cross section (sq. mm)		
	interior	exterior	TOTAL	interior	exterior	TOTAL
1	810000	640000	8020000	8893	2560	43386
2	810000	640000	8020000	7126	2560	39852
3	810000	640000	8020000	6361	2560	38322
4	810000	640000	8020000	7644	2560	40888
5	810000	640000	8020000	11208	4827	70686
6	640000	490000	6180000	13497	6739	94384
7	640000	490000	6180000	8844	4184	59528
8	640000	490000	6180000	4451	1960	28502
9	640000	490000	6180000	2560	1960	24720
10	640000	490000	6180000	2560	1960	24720
11	490000	360000	4580000	5913	4742	59246
12	490000	360000	4580000	3549	3736	44458
13	490000	360000	4580000	863	3371	35436
14	360000	250000	3220000	5444	5890	69788
15	360000	250000	3220000	2734	4370	49168
16	360000	250000	3220000	1440	3999	42870
17	250000	160000	2100000	3322	5096	57604
18	250000	160000	2100000	1000	4061	42610
19	250000	160000	2100000	1000	3184	33840
20	250000	160000	2100000	1000	5093	52930
Total concrete area			102800000	Total steel area		952938

Bill of quantities for columns of a 5 story structure
 $f_c=45$ Mpa $f_y=460$ Mpa span length= 7m

Floor level	Column cross section (sq. mm)			Reinforcement cross section (sq. mm)		
	interior	exterior	TOTAL	interior	exterior	TOTAL
1	360000	250000	3220000	1440	1000	12880
2	360000	250000	3220000	1440	2506	27940
3	360000	250000	3220000	1440	1882	21700
4	360000	250000	3220000	1440	2183	24710
5	360000	250000	3220000	1440	4245	45330
Total concrete area			16100000	Total steel area		132560

Bill of quantities for columns of a 15 story structure
 $f_c=45$ Mpa $f_y=460$ Mpa span length= 7m

Floor level	Column cross section (sq. mm)			Reinforcement cross section (sq. mm)		
	interior	exterior	TOTAL	interior	exterior	TOTAL
1	640000	490000	6180000	5593	1960	30786
2	640000	490000	6180000	2938	1960	25476
3	640000	490000	6180000	2560	1960	24720
4	640000	490000	6180000	2560	1960	24720
5	640000	490000	6180000	6472	3586	48804
6	490000	360000	4580000	7998	4880	64796
7	490000	360000	4580000	3283	3258	39146
8	490000	360000	4580000	1960	2552	29440
9	490000	360000	4580000	1960	1947	23390
10	490000	360000	4580000	1960	2372	27640
11	360000	250000	3220000	1640	3532	38600
12	360000	250000	3220000	1440	2961	32490
13	360000	250000	3220000	1440	3417	37050
14	250000	160000	2100000	1000	3051	32510
15	250000	160000	2100000	1000	4930	51300
Total concrete area			67660000	Total steel area		530868

Bill of quantities for columns of a 10 story structure
 $f_c=45$ Mpa $f_y=460$ Mpa span length= 7m

Floor level	Column cross section (sq. mm)			Reinforcement cross section (sq. mm)		
	interior	exterior	TOTAL	interior	exterior	TOTAL
1	490000	360000	4580000	1960	1440	18320
2	490000	360000	4580000	1960	1440	18320
3	490000	360000	4580000	1960	1440	18320
4	490000	360000	4580000	1960	1440	18320
5	490000	360000	4580000	1960	2683	30750
6	360000	250000	3220000	1645	2755	30840
7	360000	250000	3220000	1440	3146	34340
8	360000	250000	3220000	1440	3154	34420
9	360000	250000	3220000	1440	2700	29880
10	360000	250000	3220000	1440	4584	48720
Total concrete area			39000000	Total steel area		282230

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